GENERAL NOTES:

1. THE GENERAL CONTRACTOR AND THEIR SUBCONTRACTORS SHALL COORDINATE STRUCTURAL DRAWINGS WITH ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS, INCLUDING THE SIZE AND LOCATION OF MISCELLANEOUS ITEMS AFFECTING THE STRUCTURAL WORK SUCH AS SMALL OPENINGS, PIPE SLEEVES, RECESSES, BENT PLATES, ETC. PROMPTLY NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR OMISSIONS. OPENINGS THROUGH BEAMS, GIRDERS AND/OR COLUMNS SHALL BE VERIFIED BY ENGINEER.
2. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING SITE CONDITIONS PRIOR TO COMMENCING WORK. PROMPTLY NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES BETWEEN ACTUAL SITE CONDITIONS AND THE CONTRACT DOCUMENTS.
3. THE CONTRACTOR SHALL VERIFY ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT DIMENSIONS AND WEIGHTS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND REVIEWED SHOP DRAWINGS.
4. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. THE ERECTION PROCEDURE AND SEQUENCE INCLUDING THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, RE-SHORING, TEMPORARY SUPPORTS, ETC., ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
5. DO NOT SCALE DRAWINGS. ALL WORK REQUIRING MEASURING SHALL BE DONE ACCORDING TO FIGURES ON DRAWING. ANY MISSING DIMENSIONS WILL BE FURNISHED UPON REQUEST.
6. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL ALSO APPLY FOR ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
7. THESE GENERAL NOTES APPLY WHERE OTHER PROVISIONS ARE NOT PROVIDED BY THE DRAWINGS, SPECIFICATIONS OR TYPICAL DETAILS. IN CASE OF SPECIAL CONDITIONS INDICATED ON DRAWINGS, THE DRAWINGS SHALL GOVERN OVER THE SPECIFICATIONS.
8. THE CONTRACTOR SHALL PROVIDE ALL CENTERLINE-TO-CENTERLINE DIMENSIONS TO THE STEEL FABRICATOR PRIOR TO SHOP DRAWING SUBMITTAL AND FABRICATION OF STRUCTURAL STEEL.
9. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING ALL SHORING, BRACING, AND STRUCTURAL SUPPORTS. TEMPORARY SHORING: PROVIDE AND MAINTAIN SHORING, BRACING, AND STRUCTURAL SUPPORTS AS REQUIRED TO PRESERVE STABILITY AND PREVENT MOVEMENT, SETTLEMENT, OR COLLAPSE OF CONSTRUCTION AND FINISHES TO REMAIN AS WELL AS ADJACENT EXISTING STRUCTURES, AND TO PREVENT UNEXPECTED OR UNCONTROLLED MOVEMENT OR COLLAPSE OF CONSTRUCTION BEING DEMOLISHED.
10. CONSTRUCTION SHALL BE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL ORDINANCES, AND THE INTERNATIONAL BUILDING CODE 2024.

DESIGN NOTES

REFERENCED CODES

GEORGIA RESIDENTIAL BUILDING CODE	GARC	2024
INTERNATIONAL RESIDENTIAL BUILDING CODE	IRC	2024
MINIMUM DESIGN LOADS	ASCE-7	2022
MASONRY STRUCTURES	TMS 402/602	2022
STRUCTURAL CONCRETE	ACI 318	2025
STRUCTURAL STEEL	AISC 360	2022

DESIGN LOADS

OCCUPANCY OR USE	UNIFORM LIVE/DEAD (psf)
ROOF*	20 / 20
FLOOR	40 / 20

WIND DESIGN CRITERIA

BASIC WIND SPEED (V_{ult}): FACTORED WIND SPEED (V_{asd}):	105 mph 82 mph
RISK CATEGORY:	II
WIND EXPOSURE CLASSIFICATION:	B
INTERNAL PRESSURE COEFFICIENT:	±0.18

*REFER TO PROVIDED DIAGRAMS AND TABLES FOR COMPONENTS & CLADDING PRESSURES

SEISMIC DESIGN CRITERIA

RISK CATEGORY:	II
SEISMIC IMPORTANCE FACTOR (I_s):	$I_s = 1.0$
MAPPED SPECTRAL RESPONSE ACCELERATIONS:	$S_s = 0.24g$ $S_1 = 0.092g$
SITE CLASS:	D (ASSUMED)
MAPPED SPECTRAL RESPONSE ACCELERATIONS:	$S_{ds} = 0.2g$ $S_{d1} = 0.13g$
SEISMIC DESIGN CATEGORY:	C
BASIC SEISMIC-FORCE RESISTING SYSTEM(S):	LIGHT FRAME WALLS SHEATHED WITH WOOD PANELS RATED FOR SHEAR RESISTANCE
DESIGN BASE SHEAR:	Fx = Fy = 2.6k
RESPONSE MODIFICATION FACTOR(S):	R = 6.5
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE

SNOW DESIGN CRITERIA

SLOPED ROOF SNOW LOAD; P_s	11.6 psf
WINTER WIND PARAMETER; W_2	0.35
SNOW EXPOSURE FACTOR; C_e	0.9
SLOPE FACTOR; C_s	1.0
SNOW IMPORTANCE FACTOR; I_s	1.0
THERMAL FACTOR; C_t	1.18
DESIGN FROST DEPTH	12"

*REFER TO PROVIDED DIAGRAMS FOR SNOW DRIFT

ABBREVIATION (ALPHABETIZED):

A.B.	ANCHOR BOLTS
ARCH.	ARCHITECT
C.J.	CONTROL JOINT
C.I.P.	CAST IN PLACE
CFR	CARBON FIBER REINFORCEMENT
CONC.	CONCRETE
CONT.	CONTINUOUS
CONX.	CONNECTION
COORD.	COORDINATE
D&E	DRILL & EPOXY
E.O.C.	EDGE OF CONCRETE
E.W.	EACH WAY
EX.	EXISTING
EXP.	EXPANSION
EXT.	EXTERIOR
F.F.E.	FINISH FLOOR ELEVATION
FLR.	FLOOR
FTG.	FOOTING
GPR	GROUND-PENETRATING RADAR
GYP.	GYPSUM BOARD
H.R.	HANDRAIL
HDR.	HEADER
HORZ.	HORIZONTAL
INT.	INTERIOR
MAX.	MAXIMUM
MANF.	MANUFACTURER
M.C.	MOMENT CONNECTION
MIN.	MINIMUM
O.C.	ON CENTER
O.W.T.	OPEN WEB TRUSS
P.A.F.	POWDER ACTUATED FASTENER
REINF.	REINFORCEMENT
REQ.	REQUIRED
SIM.	SIMILAR
S.O.G.	SLAB ON GRADE
STD.	STANDARD
STL.	STEEL
SQ.	SQUARE
T&B	TOP & BOTTOM
TYP.	TYPICAL
U.N.	UNLESS NOTED
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.W.M.	WELDED WIRE MESH

FOUNDATION:

1. FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. THIS VALUE SHALL BE VERIFIED BY A REGISTERED SOIL ENGINEER PRIOR TO FOUNDATION CONSTRUCTION. IF ACTUAL VALUES VARY BY MORE THAN TEN PERCENT FROM DESIGN BEARING PRESSURE, FOOTINGS SHALL BE REDESIGNED. ALL FOOTINGS ARE TO BE PLACED ON UNDISTURBED ORIGINAL SOIL OR COMPACTED FILL.
2. ALL BACKFILLING SHALL BE ACCOMPLISHED USING MATERIAL CONSISTING OF CRUSHED STONE AND/OR MATERIAL APPROVED BY A REGISTERED SOILS ENGINEER. FILL MATERIAL TAKEN FROM SITE EXCAVATION SHALL HAVE OPTIMUM MOISTURE CONTENT FOR COMPACTION AND BE FREE OF ANY DEBRIS. BACKFILL SHALL BE COMPACTED TO 95% MAXIMUM DENSITY AS DETERMINED BY ASTM D698, IN MAXIMUM LIFTS OF EIGHT INCHES. NO BACKFILL MATERIAL SHALL BE PLACED AGAINST WALLS WITHOUT PROVISIONS FOR ADEQUATE BRACING OF THESE WALLS.
3. FILL MATERIAL SHALL BE ADEQUATELY DRAINED TO PREVENT ANY DAMAGE TO THE BUILDING'S FOUNDATION BY GROUND WATER FLOWS OR SURFACE WATER RUN-OFF.
4. REMOVE ORGANIC MATERIALS AND LARGE ROCKS AND PROVIDE TERMITE TREATMENT PRIOR TO PLACING VAPOR BARRIER AND CONCRETE SLABS.
5. ALL SOIL CONDITIONS ARE SUBJECT TO EVALUATION BY A SOILS ENGINEER PRIOR TO FOUNDATION CONSTRUCTION. SOILS WITHOUT ADEQUATE CAPACITY TO SUPPORT DESIGN LOADS MUST BE REPLACED OR MODIFIED PRIOR TO PROCEEDING WITH CONSTRUCTION.
6. SLAB TO BE PLACED ON VAPOR RETARDER ON 4" OF #57 STONE OR COMPACTED SAND OR CRUSHER RUN. VAPOR RETARDER MAY BE OMITTED FROM DRIVES, WALKS, PATIOS, AND OTHER FLAT WORK NOT LIKELY TO BE ENCLOSED AND HEATED AT A LATER DATE. JOINTS LAPPED NOT LESS THAN 6".
7. ALL VAPOR RETARDERS TO BE MINIMUM 10MIL THICK AND CONFORM TO THE ASTM E1745 CLASS A UNLESS NOTED OTHERWISE WITH JOINTS LAPPED NOT LESS THAN 6". PLACE BETWEEN CONCRETE FLOOR SLAB AND GRANULAR BASE. PERMEABILITY SHALL BE CHOSEN ACCORDING TO THE ACI 302.2R-22. INSTALLATION OF VAPOR RETARDERS SHALL COMPLY WITH ASTM E1643-18A.
8. ALL WIRE MESH AND REBARS ARE TO BE PLACED ON SUPPORTS PER ACI STANDARDS PRIOR TO POURING CONCRETE.
9. SEE ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHING REQUIREMENT.
10. WHERE A UTILITY LINE PASSES UNDER A FOOTING, PROVIDE A STEEL OR PRECAST SLEEVE WITH MINIMUM 2" CLEAR ON ALL SIDES OF PIPE. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD THE OVERALL THICKNESS OF THE SLAB, SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 2½" MUST BE MAINTAINED OVER EMBEDDED ITEMS.
11. UNLESS NOTED OTHERWISE, CONCRETE WALLS ARE DESIGNED AS VERTICALLY SPANNING WALLS, NOT AS CANTILEVERED RETAINING WALLS. THE GENERAL CONTRACTOR SHALL PROVIDE, INSTALL, AND MAINTAIN TEMPORARY SHORING/BRACING AS REQUIRED DURING BACKFILLING AND COMPACTION OF RETAINED GRADE. WALLS SHALL BE CURED AT LEAST SEVEN DAYS PRIOR TO PLACEMENT OF BACKFILL.
12. ANCHOR BOLTS ARE TO BE GRADE F1554, GR 36 AND EMBED 7" INTO THE FOUNDATION, AND PROJECT 4" MIN UNLESS NOTED OTHERWISE.

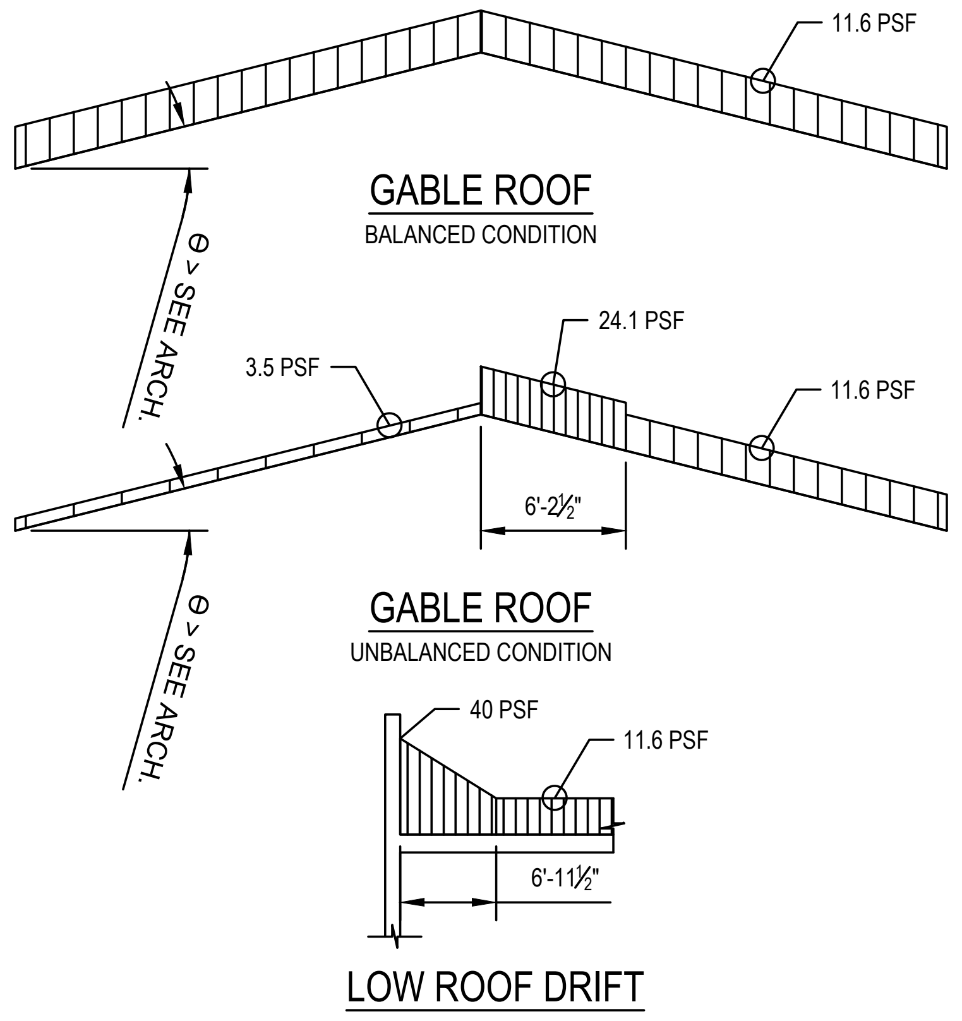
REINFORCING:

1. CONCRETE REINFORCING STEEL SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A615, GRADE 40 FOR #3 BARS AND ASTM A615, GRADE 60 FOR #4 AND LARGER BARS. WELDED WIRE MESH SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A185.
2. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI 315, LATEST EDITION.
3. REINFORCING STEEL SHALL BE SPLICED ONLY AS INDICATED ON THE PLANS. WHEN SPLICE LENGTHS ARE NOT GIVEN ON THE PLANS, THEY SHALL BE TAKEN FROM THE TABLE BELOW. USE "CLASS B" LAPS UNLESS THE PLANS INDICATE "CLASS A".

BAR SIZE	CLASS "B" SPLICE	CLASS "A" SPLICE
#3	28"	22"
#4	37"	29"
#5	47"	36"

LAPS SHOWN ABOVE WERE CALCULATED PER ACI 318-25 EQ.25.4.2.4a FOR MATS, WALLS, BEAMS, COLUMNS AND SLABS. VALUES ASSUMED ARE: f'c 3000 PSI, Ktr=0, 1" MIN COVER AND 2" MIN CLEAR BETWEEN BARS FOR #4, #5 AND #6 BARS, AND 1½" MIN COVER AND 3" MIN CLEAR BETWEEN BARS FOR #7 THRU #11 BARS. SHORTER LAPS MAY BE CALCULATED FOR SOME SPECIFIC CONDITIONS SUCH AS TIED BEAMS OR ADDITIONAL COVER. LAPS MUST BE INCREASED 50% PER ACI 318-25 FOR EPOXY COATED REBAR, OR 30% FOR LIGHTWEIGHT CONCRETE.

4. LAPS IN W.W.F. SHOULD BE ONE MESH PLUS TWO INCHES AT SPLICES.
5. PROVIDE EXTRA REINFORCING AROUND ALL OPENINGS, INCLUDING DOOR OPENINGS: TWO #5 BARS AT ALL FOUR SIDES OF EACH OPENING AND EXTEND 2'-0" BEYOND THE CORNERS OF THE OPENING. ADD TWO #5 BARS 4'-0" LONG AS DIAGONAL BARS AT EACH CORNER.
6. TEMPERATURE STEEL FOR SLABS SHALL BE #3 @ 16" O.C. OR 6x6-W2.9xW2.9 W.W.M. PLACED 1" FROM TOP OF SLAB UNLESS SHOWN OTHERWISE. WHERE W.W.M. IS USED, USE PLASTIC MESH 'CHAIRS' SPACED SO THAT NO MORE THAN 3 SQ FEET OF MESH IS SUPPORTED ON EACH 'CHAIR', UNLESS OTHERWISE SPECIFIED. ALTERNATELY, SLAB MAY BE REINFORCED WITH MACROFIBER (AT 3.0 LB PER CY), FORTA-FERRO OR FIBERMESH650 OR EQUIVALENT.
7. ALL BAR HOOKS SHALL BE STANDARD 90-DEGREE HOOKS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
8. SEE CHAPTER 25 OF ACI 318-25 FOR CONDITIONS NOT NOTED.
9. DESIGN OF STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318, LATEST EDITION.



A
S-0.0
SNOW DRIFT DIAGRAMS
SCALE: N.T.S.

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, "SPECIFICATION FOR STRUCTURAL CONCRETE", EXCEPT AS NOTIFIED BY THE REQUIREMENTS OF THESE STRUCTURAL DRAWINGS.
2. CONCRETE SHALL BE NORMAL WEIGHT AND HAVE A DESIGNATED COMPRESSIVE STRENGTH (F'c) IN 28 DAYS OF 3000 PSI WITH A 4" (±1") SLUMP UNLESS NOTED OTHERWISE. REFER TO CONCRETE MIX SCHEDULE FOR ADDITIONAL CONCRETE STRENGTH AND DURABILITY REQUIREMENTS.
3. SAWN CONTROL JOINTS IN FLOOR SLABS, AS INDICATED ON THE PLANS BY "C.J.", SHALL BE 1/8" WIDE x 1" DEEP AND SHOULD BE CUT (AS SOON AS CONDITIONS ALLOW) WITHIN 12 HOURS AFTER CONCRETE IS PLACED. MAXIMUM "C.J." SPACING SHALL BE 15 FT IN EITHER DIRECTION.
4. CONSTRUCTION OR ISOLATION JOINTS SHALL BE PLACED IN THE SLAB AT A MAXIMUM SPACING OF 150 FEET IN BOTH DIRECTIONS.
5. JOINT FILLER AT CONTROL JOINTS, CONSTRUCTION AND ISOLATION JOINTS SHALL BE A SEMI-RIGID MATERIAL AND WILL BE APPLIED FULL DEPTH TO PREVENT MOVEMENT.
6. SLAB-ON-GRADE NOT OTHERWISE SPECIFIED, SHALL BE 4" THICK MINIMUM. SEE REINFORCING AND FOUNDATION NOTES FOR SLAB REINFORCEMENT & UNDERLAYMENT REQUIREMENTS.
7. CONCRETE COARSE AGGREGATE, WITH A MAXIMUM SIZE OF 1" MAY BE USED IN FOUNDATIONS. ALL OTHER CONCRETE SHALL HAVE A COARSE AGGREGATE WITH A MAXIMUM SIZE OF ¾".
8. CONCRETE MIXING, TRANSPORTING, PLACING, AND CURING SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF ACI 301. READY-MIXED CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH REQUIREMENTS OF ASTM C94 OR ASTM C685.
9. ALL EQUIPMENT FOR MIXING AND TRANSPORTING CONCRETE SHALL BE CLEAN. ALL DEBRIS, WATER AND ICE SHALL BE REMOVED PRIOR TO PLACING CONCRETE. FORMS SHALL BE PROPERLY COATED. MASONRY FILLER UNITS THAT WILL BE IN CONTACT WITH CONCRETE SHALL BE WELL DRENCHED. REINFORCEMENT SHALL BE CLEAN OF ICE OR OTHER DELETERIOUS COATING. ALL LAITANCE AND OTHER UNSOUND MATERIAL SHALL BE REMOVED BEFORE ADDITIONAL CONCRETE IS PLACED AGAINST HARDENED CONCRETE.
10. SAMPLES FOR STRENGTH TESTS SHALL BE TAKEN IN ACCORDANCE WITH ASTM C172. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 150 CU YD OF CONCRETE NOR LESS THAN ONCE FOR EACH 5,000 SQ FT OF SURFACE AREA FOR SLABS OR WALLS. A STRENGTH TEST SHALL BE THE AVERAGE OF THE STRENGTHS OF TWO CYLINDERS MADE FROM THE SAME SAMPLE OF CONCRETE AND TESTED AT 28 DAYS OR AT THE TEST AGE DESIGNATED FOR DETERMINATION OF F'c.
11. CYLINDERS FOR STRENGTH TESTS SHALL BE MOLDED AND LABORATORY CURED IN ACCORDANCE WITH ASTM C31 AND TESTED IN ACCORDANCE WITH ASTM C39.
12. NO CONSTRUCTION LOADS SHALL BE SUPPORTED ON, NOR ANY SHORING REMOVED FROM, ANY PART OF THE STRUCTURE UNDER CONSTRUCTION EXCEPT WHEN THAT PORTION OF THE STRUCTURE IN COMBINATION WITH REMAINING FORMING AND SHORING SYSTEM HAS SUFFICIENT STRENGTH TO SUPPORT SAFELY ITS WEIGHT AND LOADS PLACED THEREON.
13. THE CLEAR DISTANCE BETWEEN REINFORCING BARS, BUNDLED BARS, PRE-STRESSING TENDONS, AND DUCTS SHALL BE IN ACCORDANCE WITH THE LIMITATIONS OF ACI 318.
14. MINIMUM COVER FOR CAST-IN-PLACE CONCRETE REINFORCEMENT:

	MINIMUM COVER (IN)
(a) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
(b) CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2
#5 BAR, W31 OR D31 WIRE, AND SMALLER	1½
(c) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLAB, WALLS, JOIST: #11 BARS AND SMALLER	¾
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1½

15. UNLESS NOTED OTHERWISE, PROVIDE VERTICAL CONTROL JOINTS IN STEP WALLS AND RETAINING WALLS @ 25'-0" O.C. MAXIMUM.

MINIMUM REQUIRED SUBMITTALS

	PRODUCT LITERATURE	SHOP DRAWINGS	CALCULATIONS (SIGNED & SEALED)
EPOXY & MECHANICAL ANCHORS	✓		
MISC. CONCRETE PRODUCTS	✓		
CONCRETE MIX DESIGNS	✓		
CONCRETE REINF. STEEL		✓	
MASONRY PRODUCTS	✓		
WOOD TRUSSES		✓	✓

* PROVIDE CALCULATIONS IF DELEGATED DESIGN IS USED FOR ANY ITEMS NOT SHOWN IN OR DEVIATING FROM DESIGNS SHOWN IN THESE DRAWINGS.

SUBMITTAL NOTES:

1. THE GENERAL CONTRACTOR SHALL SUBMIT A SCHEDULE OF SUBMITTALS PRIOR TO CONSTRUCTION BEGINNING ON THE PROJECT. THE SCHEDULE SHOULD DESCRIBE WHAT EACH SUBMITTAL IS, WHETHER IT IS THE ENTIRE PACKAGE, OR BROKEN INTO PHASES FOR REVIEW.
2. THE GENERAL CONTRACTOR SHALL ALLOW FOR 10 BUSINESS DAYS OF ALL STRUCTURAL SUBMITTALS. IF THE CONTRACTOR WISHES TO EXPEDITE OR REDUCE THE REVIEW TIME, IT SHALL BE DONE FOR AN ADDITIONAL CHARGE AND MUST BE NEGOTIATED PRIOR TO THE SUBMISSION OF THE PACKAGE.
3. IN THE EVENT THAT THE CONTRACTOR ATTEMPTS TO SUBMIT THE ENTIRE PROJECT AT THE SAME TIME FOR SUBMISSION, ADDITIONAL TIME WILL BE REQUIRED BEYOND THE STANDARD 10 DAY REVIEW TIME.
4. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM-OF-RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION."

WOOD:

1. ALL CONVENTIONAL TIMBER CONSTRUCTION SHALL CONFORM TO THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" 2024 BY THE AMERICAN WOOD COUNCIL. ALL STUDS SHALL BE #2 SPRUCE-PINE-FIR OR BETTER. ALL OTHER TIMBER SHALL BE STRUCTURAL GRADED #2 SOUTHERN PINE OR BETTER UNLESS NOTED OTHERWISE. PLYWOOD CONSTRUCTION SHALL UTILIZE AMERICAN PLYWOOD ASSOCIATION RATED MATERIALS.
2. ALL TIMBER IN CONTACT WITH CONCRETE OR MASONRY WITHIN 6" OF GRADE, OR REMAIN EXPOSED TO WEATHER SHALL BE PRESSURE TREATED #2 SOUTHERN PINE, AWPA STANDARD U1.
3. AT LOAD BEARING WALLS, TOP PLATE SHALL BE DOUBLE, SILL PLATE SHALL BE SINGLE. ALL LOAD BEARING WALLS SHALL BE CONSTRUCTED OF STUDS AND PLATES WITH A MOISTURE CONTENT ≤ 19%.
4. ROOF SHEATHING SHALL BE 24/16 ⅝" MIN. APA RATED SHEATHING U.N.O., LAID WITH FACE GRAIN PERPENDICULAR TO THE FRAMING AND STAGGERED 4'-0". MINIMUM NAILING SHALL BE 8d NAILS @ 6" O.C. ALL UNSUPPORTED EDGES OF PLYWOOD SHEATHING SHALL BE SUPPORTED WITH SIMPSON PSCL CLIPS, PROVIDE (1) CLIP EQUALLY SPACED BETWEEN EACH TRUSS/SUPPORT. CLIPS NOT REQUIRED FOR TONGUE AND GROOVE SHEATHING.
5. SUB-FLOOR SHEATHING SHALL BE 48/24 ¾" APA RATED PLYWOOD U.N.O. GLUED AND NAILED TO JOISTS. MINIMUM NAILING SHALL BE 8d NAILS AT 6" O.C. ON PANEL EDGES AND 12" O.C. AT INTERIOR SUPPORTS.
6. ALL EXTERIOR WALL SHEATHING AND SHEAR PANEL SHEATHING SHALL BE ½" PLYWOOD U.N.O. MINIMUM NAILING SHALL BE 8d x 2½" LONG NAILS SPACED AT 4" O.C. ON PANEL EDGES AND 12" O.C. AT INTERIOR AREAS. CONTRACTOR TO INSPECT ALL NAILING BEFORE EXTERIOR VENEER IS PLACED. DRYWALL NAILING TO BE APPROVED BEFORE TAPING. PROVIDE STEEL PLATE WASHERS @ EXTERIOR WALL AND SHEAR PANEL ANCHOR BOLTS, SEE NOTE BELOW FOR TYPICAL BOLT SIZE & SPACING. AT 2x4 WALLS, WASHERS SHALL BE MINIMUM 3"x3"x¼" STEEL PLATE (SIMPSON BPS ½-3 OR EQUIVALENT). AT 2x6 WALLS, WASHERS SHALL BE MINIMUM 3"x4½"x¼" STEEL PLATE (SIMPSON BPS ½-6 OR EQUIVALENT) ORIENTED WITH LONG DIRECTION PERPENDICULAR TO WALL.
7. ALL LOAD BEARING AND EXTERIOR WALLS SHALL BE AS FOLLOWING (U.N.O.):
   EXTERIOR: 2x6 @ 16" O.C.
   INTERIOR: 2x4 (MIN.) @ 16" O.C.
8. PLACE (2) STUDS MINIMUM IN WALLS FOR BEARING OF ALL BEAM, HIP, VALLEY MEMBERS U.N.O AS GREATER ON PLANS. EACH LAYER OF GANGED STUDS SHALL BE NAILED TO ADJACENT STUDS WITH 10d NAILS @ 8" O.C. STAGGERED. CONTINUE ALL POSTS DOWN TO FOUNDATION OR TRANSFER BEAMS BELOW.
9. BEAMS DESIGNATED ON THE PLANS AS LVL SHALL BE 1¾" WIDTH 2.0E LAMINATED VENEER LUMBER AS MANUFACTURED BY TRUS-JOIST (WEYERHAUSER) OR APPROVED ALTERNATE.
10. COLUMNS DESIGNATED ON THE PLANS AS PSL SHALL BE 1.8E PARALLEL STRAND LUMBER AS MANUFACTURED BY TRUS-JOIST (WEYERHAUSER) OR APPROVED ALTERNATE.
11. DURING CONSTRUCTION, PROVIDE BRACING FOR FRAMING UNTIL ALL ELEMENTS FOR EXTERIOR SHEAR WALLS AND FLOOR DIAPHRAGMS ARE IN PLACE.
12. UNLESS SPECIFICALLY DETAILED OTHERWISE, USE A METAL CONNECTOR AT ALL BEAM & TRUSS SUPPORTS, ROOF RAFTER HOLD DOWNS (HURRICANE CLIPS), BEAM TO BEAM, POST TO BEAM, AND POST TO FLOOR CONNECTIONS. BASIS OF DESIGN METAL FRAMING CONNECTOR PRODUCTS ARE SIMPSON STRONG-TIE OR MITEK USP, OR AN APPROVED EQUAL, SIZED ACCORDING TO MANUFACTURERS' RECOMMENDATIONS FOR CONNECTION TYPE AND LOADS ENCOUNTERED. FOLLOW ALL MANUFACTURER INSTALLATION INSTRUCTIONS WITH THE NUMBER & TYPE OF FASTENERS REQUIRED TO ACHIEVE THE MAXIMUM LOAD CAPACITY UNLESS NOTED OTHERWISE.
13. PROVIDE ½"Ø ANCHOR BOLT W/ NUT & WASHER AT SILL PLATE, @ 4'-0" O.C. & 7" MIN. EMBEDMENT. PROVIDE (2) A.B. MIN. PER SILL PLATE SEGMENT W/ (1) A.B. LOCATED @ 4" MIN. & 12" MAX. FROM ENDS.
14. ALL NAILS REFERENCED IN THE DRAWINGS (UNLESS NOTED OTHERWISE) ARE COMMON WIRE NAILS TO BE SIZED AS FOLLOWS:
   8d = 0.131"Ø
   10d = 0.148"Ø
   16d = 0.162"Ø

WOOD STUD WALL FRAMING:

1. 2x4 AND 2x6 STUD MATERIAL:

COMMERCIAL GRADE	#2
SPECIES	SPRUCE-PINE-FIR
GRADING RULES AGENCY	NLGA
MOISTURE CONTENT	≤ 19%

(Fb=875 psi; Fc parallel=1150 psi; E=1400 ksi)

2. MID-POINT HORIZONTAL BLOCKING: PROVIDE MIN. ONE ROW OF HORIZONTAL BLOCKING @ MIDPOINT OF STUD WALL.
3. LOAD BEARING STUD WALLS TO HAVE 2x4 OR 2x6 DOUBLE TOP PLATES AND 2x4 OR 2x6 SINGLE BOTTOM PLATES. MATERIAL FOR PLATES TO BE NO. 2 SOUTHERN PINE PT FOR SILL PLATE IN CONTACT WITH FOUNDATION, AND SPF NO. 2 GRADE FOR PLATES ABOVE GROUND LEVEL. PROTECT ALL WOOD FRAMING FROM MOISTURE ON THE JOBSITE. IT IS RECOMMENDED THAT ALL HORIZONTAL PLATES HAVE A MOISTURE CONTENT OF 15% OR LESS TO REDUCE SHRINKAGE EFFECTS.
4. NON-LOAD BEARING WALLS THAT ARE NOT SHEAR WALLS SHALL BE UTILITY GRADE SPF OR BETTER @ 24" O.C. MAX. WITH SINGLE UTILITY TOP & SINGLE UTILITY BOTTOM PLATE. NON-LOAD BEARING WALLS SILL PLATE IN CONTACT WITH FOUNDATION SHALL BE NO. 3 SOUTHERN PINE PT. NON-BEARING PARTITION WALLS SHALL NOT BE FRAMED TIGHT TO THE UNDERSIDE OF JOISTS, FASTEN TO FRAMING ABOVE WITH SIMPSON DEFLECTION SCREWS PER MANUF.
5. FINGER-JOINTED STUDS WITH 1½"x3½" OR 1½"x5½" NOMINAL DIMENSIONS MAY BE USED IN LIEU OF CONVENTIONAL 2x4 OR 2x6 STUDS RESPECTIVELY. THE STUD BASE MATERIAL SHALL MEET OR EXCEED THE GRADE REQUIREMENTS OF NOTE 1. FINGER JOINTED STUDS USED AT EXTERIOR WALLS SHALL BE MANUFACTURED TO WITHSTAND BENDING & TENSION STRESSES (NOT MARKED "STUD USE ONLY" OR "VERTICAL USE ONLY"). ANY FINGER JOINTED STUD USED IN ANY RATED ASSEMBLY SHALL USE HEAT RESISTANT ADHESIVE AND BE MARKED WITH THE "HRA" STAMP.
6. FINGER JOINTED STUDS NOT ALLOWED WITHOUT SPECIFIC APPROVAL BY EOR.
7. ALL STUD POSTS, AND STUDS INDICATED IN THIS SCHEDULE, ARE DESIGNED WITH THE ASSUMPTION THAT SHEATHING (EITHER GYPSUM OR OSB/PLYWOOD) IS ATTACHED TO ONE (1) 1½" WIDE FACE OF EACH STUD CONTINUOUSLY PER THE INDICATED ATTACHMENT SPACING IN THE FINAL CONSTRUCTION. PER STRUCTURAL GENERAL NOTE THE CONTRACTOR IS TO PROVIDE TEMPORARY BRACING UNTIL ALL ELEMENTS SHOWN ARE IN PLACE, HENCE CONTRACTOR SHALL WORK SCHEDULING OF SHEATHING INSTALLATION AND/OR TEMPORARY BRACING DURING CONSTRUCTION SUCH THAT STUDS ALWAYS HAVE THE CAPACITY TO SUPPORT THE DEAD AND LIVE LOADS OF CONSTRUCTION THAT ARE CURRENTLY PRESENT AND THE DESIGN LATERAL LOADS (SEE STRUCTURAL GENERAL NOTES 4 AND 9) AT ANY GIVEN TIME DURING THE CONSTRUCTION PROCESS.
8. EACH LAYER OF MULTIPLE STUDS SHALL BE NAILED TO ADJACENT STUDS WITH 10d NAILS @ 8" O.C. STAGGERED. STUD POST SIZE SHALL MATCH STUD WALL SIZE U.N.O..
9. CUTTING, NOTCHING, AND BORING OF ALL LOAD BEARING STUDS SHALL NOT EXCEED A DEPTH OF 25% OF THE STUD WIDTH (⅞" FOR 2x4 STUDS, 1⅜" FOR 2x6 STUDS), AND ALL AFFECTED STUDS SHALL BE REINFORCED WITH A SIMPSON SS STUD SHOE. NOTCHES OR BORED HOLES EXCEEDING 25% OF STUD WIDTH AND LESS THAN 50% SHALL BE REINFORCED WITH A SIMPSON HSS STUD SHOE. AS AN ALTERNATIVE TO USING STUD SHOES, AT ALL NOTCHED OR BORED STUDS THE NUMBER OF STUDS AT THE LOCATION OF THE NOTCHING OR BORING SHALL BE DOUBLED (≤ 25% NOTCH/BORED) OR TRIPLED (25%-50% NOTCH/BORED). ALL BORES SHOULD BE LOCATED AS CLOSE TO THE CENTERLINE OF THE STUD AS POSSIBLE. NO MORE THAN TWO SUCCESSIVE STUD GROUPS SHALL BE CUT, NOTCHED, OR BORED. CONTACT THE STRUCTURAL ENGINEER OF RECORD AT LOCATIONS REQUIRING LARGER NOTCHES OR BORES. POSTS SHALL NOT BE CUT, NOTCHED, OR BORED UNLESS SPECIFICALLY NOTED IN THESE DRAWINGS.
10. CUTTING AND NOTCHING OF ALL LOAD BEARING STUDS SHALL NOT EXCEED A DEPTH OF 25% OF THE STUD WIDTH (⅞" FOR 2x4 STUDS). NOTCHES EXCEEDING 25% OF STUD WIDTH AND LESS THAN 2½" SHALL BE REINFORCED WITH A SIMPSON SS STUD SHOE. CONTACT THE STRUCTURAL ENGINEER OF RECORD AT LOCATIONS REQUIRING LARGER NOTCHES.
11. BORING OF ALL STUD WALL SHALL NOT EXCEED 40% OF STUD WIDTH OR BE CLOSER THAN ⅝" FROM EITHER EDGE OF THE STUD. IF BORED HOLES BETWEEN 40% AND 60% OF STUD WIDTH, THE NUMBER OF STUDS AT THE LOCATION OF THE BORED HOLE SHALL BE DOUBLED AND NO MORE THAN TWO SUCCESSIVE STUD GROUPS SHALL BE BORED.

SHOP-FABRICATED WOOD TRUSSES:

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" 2024 AND THE "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION" 2022 BY THE TRUSS PLATE INSTITUTE.
2. TRUSS LAYOUT SHOWN IN THESE DRAWINGS IS PROVIDED FOR CONCEPTUAL DESIGN ONLY. THE MANUFACTURER AND DESIGNER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS, BOTH SIGNED AND SEALED BY A LICENSED STRUCTURAL ENGINEER (STATE OF GEORGIA). SUBMITTALS SHALL BE MADE TO THE ARCHITECT AND ENGINEER FOR REVIEW AND, IF REQUIRED, TO THE LOCAL BUILDING OFFICIAL FOR APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL INCLUDE LAYOUT PLAN AND ALL CONNECTORS NOT SPECIFIED IN THESE DRAWINGS. CALCULATIONS SHALL BE BASED ON THE SPECIFIED LOADING CONDITIONS SHOWN HEREIN. THE MANUFACTURER SHALL PROVIDE ALL CONNECTIONS BETWEEN TRUSSES AND LOAD BEARING ELEMENTS. TRUSS FABRICATOR IS TO REVIEW AND APPROVE DIMENSIONS, SHAPES AND DETAILS SHOWN ON SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT AND ENGINEER FOR REVIEW AND COMMENT.
3. FLOOR TRUSSES SHALL BE DESIGNED FOR ALL LOADS IMPOSED, INCLUDING THOSE FROM LOAD BEARING WALLS ABOVE THAT LOAD THE TRUSSES IN A PARALLEL OR PERPENDICULAR MANNER. MULTIPLY TRUSSES (GIRDER TRUSSES) ARE TO BE PROVIDED WHERE A SINGLE TRUSS CAN NOT ADEQUATELY SUPPORT THE IMPOSED LOADS.
4. ROOF TRUSSES SHALL BE SPACED AT 24" O.C. U.N.0. ON THE PLANS. PROVIDE MULTIPLE TRUSSES (GIRDER TRUSS) WHERE ONE TRUSS CANNOT SUPPORT THE SPECIFIED LOAD.
5. TRUSS MANUFACTURER AND DESIGNER SHALL PROVIDE HANGERS AND CONNECTORS THAT ARE NOT SPECIFIED ON THE DRAWINGS. CONNECTORS THAT ARE SPECIFIED HEREIN ARE BASED UPON SIMPSON "STRONG-TIE" OR MITEK "USP" WOOD CONSTRUCTION CONNECTOR PRODUCTS. SUBSTITUTIONS MAY BE MADE FOR EQUIVALENT PRODUCTS.
6. ROOF TRUSSES SHALL HAVE CONNECTORS AT EACH END TO RESIST UPLIFT FORCES. CONNECTORS SHALL BE ATTACHED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS. TRUSS MANUFACTURER SHALL NOTIFY E.O.R. WHEN UPLIFT DEMAND EXCEEDS THE CAPACITY OF THE SPECIFIED TIE (TYPICAL [H1A] [470#], GIRDER [LGT] [1,755#]).
7. TRUSS CHORDS AND WEB MEMBERS SHALL BE 2x4 (MINIMUM).
8. TRUSS DEFLECTION CRITERIA:
   ROOF TRUSSES: L/360 LIVE LOAD ONLY; L/240 TOTAL LOAD; ¾" ABSOLUTE
9. TYPICAL TRUSS DESIGN LOADS (U.N.O.):
   ROOF SYSTEM DEAD LOADS: 10 PSF TCDL 10 PSF BCDL
   ROOF SYSTEM LIVE LOADS: 20 PSF TCLL
10. MECHANICAL EQUIPMENT MAY BE PRESENT. THE LOCATION, LOADS, AND METHODS OF ATTACHMENT FOR ALL EQUIPMENT SHALL BE COORDINATED WITH THE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS AND ACCOUNTED FOR BY THE TRUSS DESIGNER.
11. ALL REQUIRED TRUSS BRIDGING & BRACING SHALL BE SHOWN ON THE SHOP DRAWINGS PREPARED BY THE FABRICATOR & DESIGNER. SPECIFICATION SHALL BE IN ACCORDANCE WITH BUILDING COMPONENT SAFETY INFORMATION (BCSI) GUIDE 2025.
12. ALL TRUSS CONNECTOR PLATES SHALL BE MANUFACTURED FROM ASTM A446-72 GRADE A GALVANIZED STEEL OF NO LESS THAN 20 GAGE THICKNESS WITH A MINIMUM YIELD OF 33,000 PSI AND AN ULTIMATE TENSILE STRENGTH OF 45,000 PSI.
13. INSTALLATION OF ALL TRUSSES SHALL BE DONE USING A SPREADER BAR WITH A THREE POINT VERTICAL PICK. CARE SHALL BE USED IN LIFTING TO PREVENT HORIZONTAL BENDING.
14. IMPROPERLY HANDLED OR STORED TRUSSES, AS NOTED ABOVE AND IN THE SPECIFICATIONS, SHALL BE REMOVED FROM THE JOBSITE AND REPLACED AT THE EXPENSE OF THE CONTRACTOR.
15. NON-BEARING PARTITION WALLS SHALL NOT BE FRAMED TIGHT TO THE UNDERSIDE OF JOISTS. PROVIDE ½" GAP WITH TRUSS DEFLECTION CLIPS OR SIMPSON SDPW SCREWS AT PERPENDICULAR WALLS AND DEFLECTION BLOCKING OR SIMPSON SDPW SCREWS @ 48" O.C. AT PARALLEL WALLS.
16. ALL PRE-ENGINEERED TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED BY THE PROJECT STRUCTURAL ENGINEER-OF-RECORD.

MASONRY:

1. MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF TMS 602, "SPECIFICATION FOR MASONRY STRUCTURES" EXCEPT AS NOTIFIED BY THE REQUIREMENTS OF THESE STRUCTURAL DRAWINGS.
2. ALL LEVELING GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3000-PSI. EXPOSED GROUT SHALL BE NON-STAINING.
3. USE TYPE 'S' MORTAR CEMENT FOR VERTICAL AND HORIZONTAL JOINTS PER ASTM C 270.
4. ALL MASONRY GROUT SHALL CONFORM WITH ASTM C 476.
5. HALF OF THE EQUIVALENT AREA OF VERTICAL REINFORCEMENT INTERRUPTED BY OPENINGS SHALL BE PLACED AT EACH SIDE OF THE OPENING.
6. PROVIDE BRICK VENEER TIES @ 24" O.C. MAX. HORIZONTALLY, 16" O.C. MAX. VERTICALLY. TIES TO BE EITHER:
   -22GA x⅞" WIDE GALVANIZED CORRUGATED TIES
   -9GA GALVANIZED WIRE TIES

PIERRE D. COIRON
No. 29213
4-10-24

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

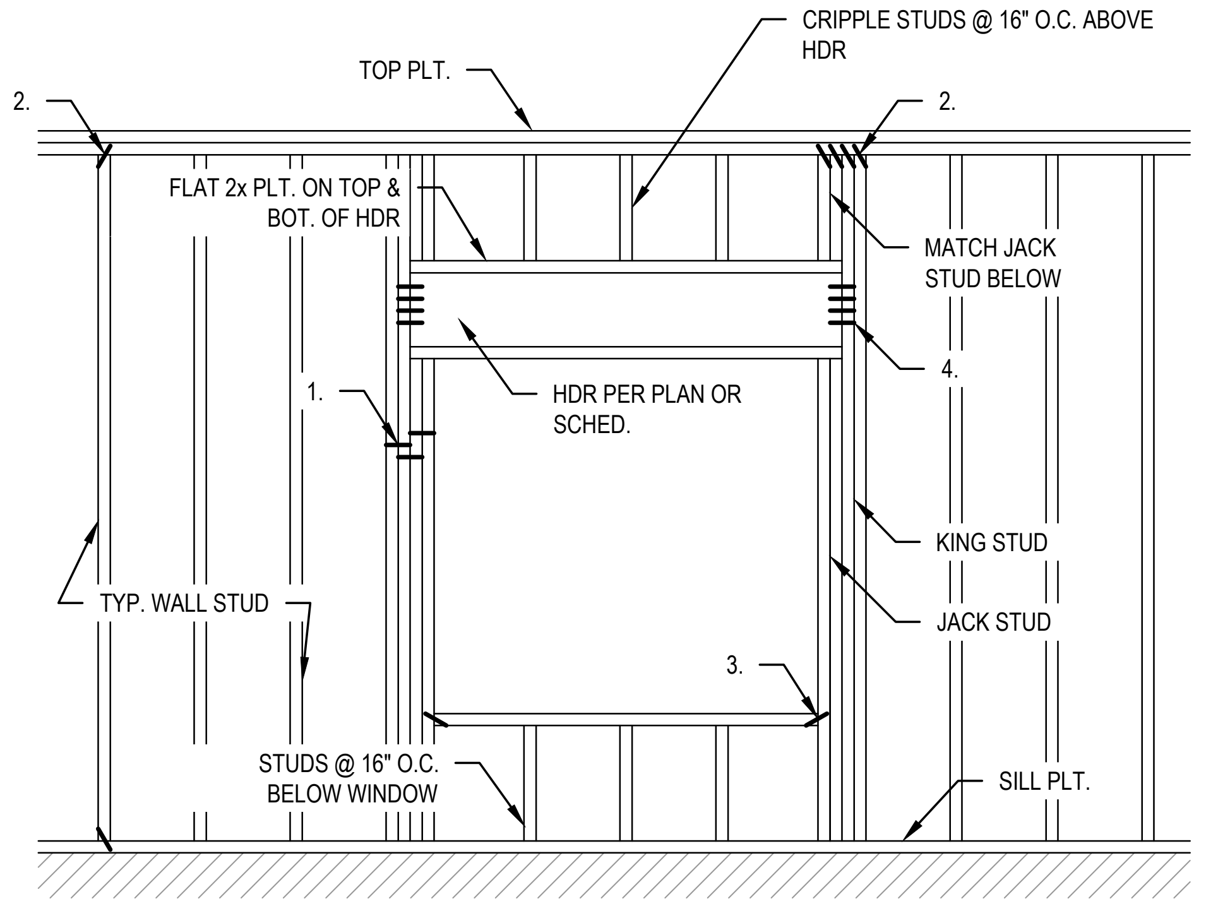
CLARATEL CARE HOMES – OPTION 2

GENERAL NOTES

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

Se STABILITY ENGINEERING
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S-0.0
PROJECT NO. SE#26180
SHT 1 OF 8
REV 0



TYP. ATTACHMENTS

1. STUD TO STUD IN BUILT-UP STUD PACK, 10d NAILS @ 8" O.C. STAGGERED EA. PLY
2. STUD TO TOP & SILL PLATES, (4) 10d TOENAILS OR (2) 16d END NAILS
3. SILL TO JAMB STUD, (4) 10d TOENAILS OR (2) 16d END NAILS
4. JAMB STUD TO HDR, (4) 10d END NAILS EA. PLY

JACK/KING STUD SCHEDULE

STUDS	JACK STUD	KING STUD
(5) GANGED STUDS	(3) STUDS	(2) STUDS
(4) GANGED STUDS	(2) STUDS	(2) STUDS
(3) GANGED STUDS	(2) STUDS	(1) STUD

WOOD HEADER SCHEDULE

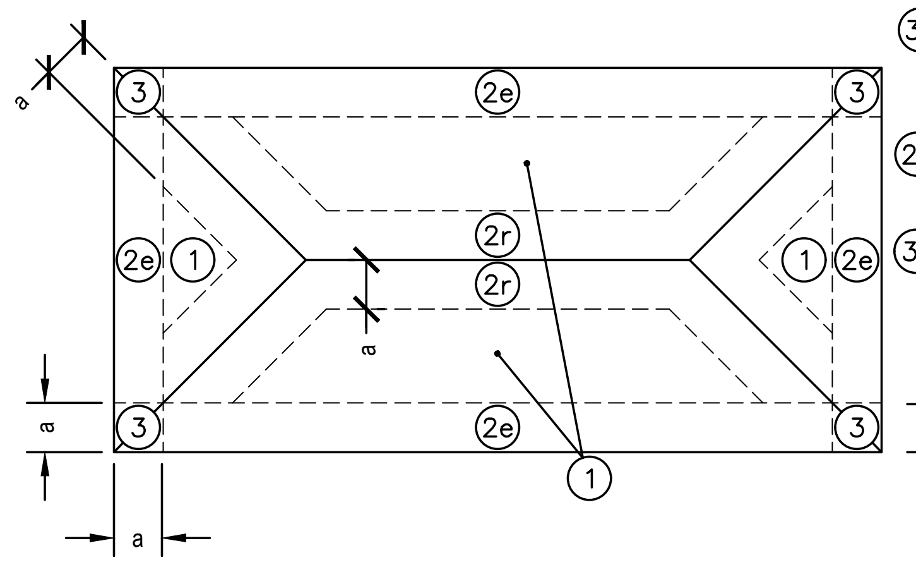
SPAN, LESS THAN	LOAD BEARING WALL	NON BEARING WALL	END POSTS @ LOAD BEARING WALL
2'-0"	(2) 2x8 OR (3) 2x6	(2) 2x6 OR (3) 2x4	P2
4'-0"	(2) 2x10 OR (3) 2x8	(2) 2x8 OR (3) 2x6	P3
6'-0"	(2) 7¼" LVL OR (3) 2x12	(2) 2x10 OR (3) 2x8	P3
8'-0"	(2) 9¼" LVL OR (3) 7¼" LVL	(2) 2x12 OR (3) 2x10	P4
10'-0"	(2) 11¼" LVL OR (3) 9¼" LVL	(2) 2x12 OR (3) 2x10	P5

NOTES:
1. USE (2) PLY HEADER AT 2x4 WALLS AND (3) PLY HEADER AT 2x6 WALLS.
2. AT MINIMUM, PROVIDE FULL-HEIGHT KING STUDS TO MATCH THE NUMBER OF STUDS REPLACED BY THE OPENING. PLACE HALF (ROUND UP) ON EACH SIDE OF THE OPENING.

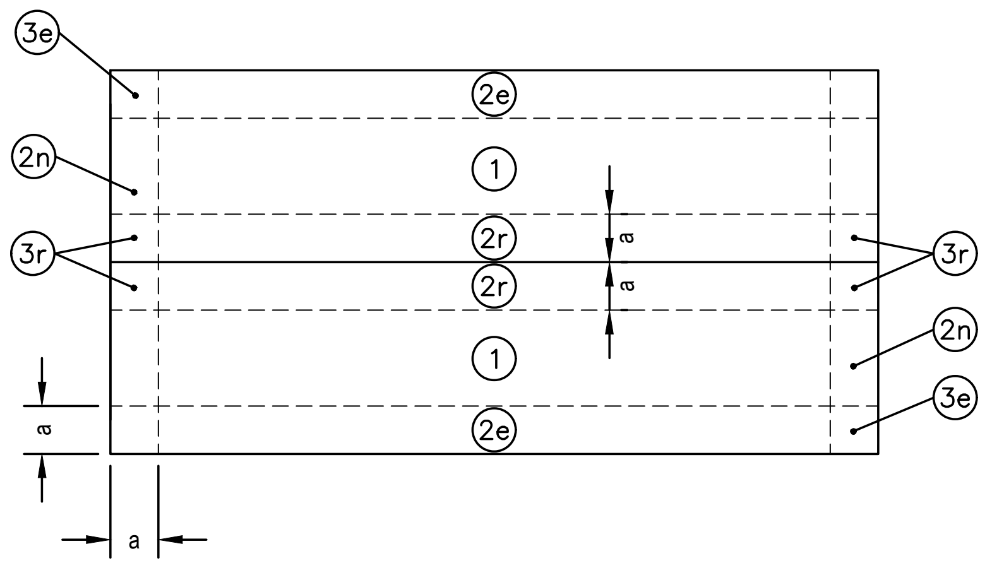
A
S-0.1

TYP. WALL OPENING FRAMING DETAIL

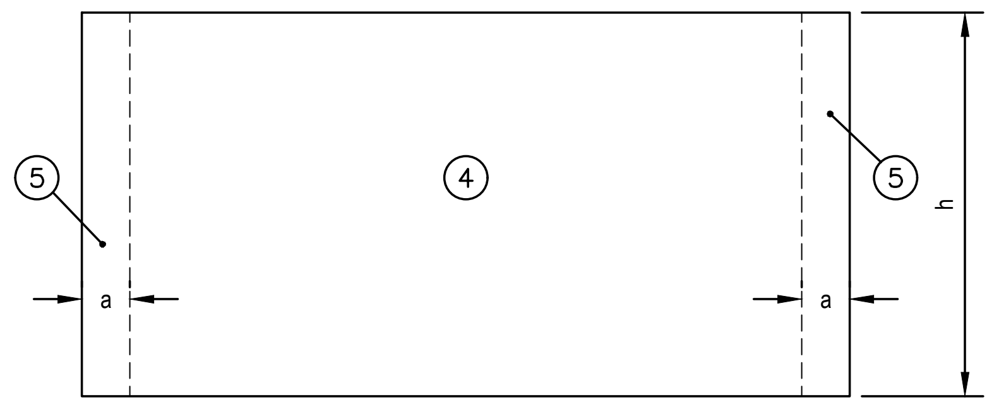
SCALE: N.T.S.



HIP ROOF 7-45 DEGREES



GABLE ROOF (7°<ϴ<45°)



WALL ELEVATION

COMPONENTS AND CLADDING PRESSURES, GABLE ROOF (PSF)						
Ae (EFFECTIVE AREA)	ZONE 1	ZONE 2e	ZONE 2n	ZONE 2r	ZONE 3e	ZONE 3r
Ae = 10 SQ. FT.	+16.0, -37.3	+16.0, -37.3	+16.0, -54.5	+16.0, -54.5	+16.0, -54.5	+16.0, -64.7
Ae = 20 SQ. FT.	+16.0, -37.3	+16.0, -37.3	+16.0, -47.1	+16.0, -47.1	+16.0, -47.1	+16.0, -55.5
Ae = 50 SQ. FT.	+16.0, -22.7	+16.0, -22.7	+16.0, -37.3	+16.0, -37.3	+16.0, -37.3	+16.0, -43.2
Ae = 100 SQ. FT.	+16.0, -11.6	+16.0, -11.6	+16.0, -30.0	+16.0, -30.0	+16.0, -30.0	+16.0, -33.9

COMPONENTS AND CLADDING PRESSURES, HIP ROOF (PSF)				
Ae (EFFECTIVE AREA)	ZONE 1	ZONE 2e	ZONE 2r	ZONE 3
Ae = 10 SQ. FT.	+16.0, -25.4	+16.0, -33.9	+16.0, -44.2	+16.0, -33.9
Ae = 20 SQ. FT.	+16.0, -25.4	+16.0, -31.2	+16.0, -39.9	+16.0, -31.2
Ae = 50 SQ. FT.	+16.0, -22.4	+16.0, -27.5	+16.0, -34.1	+16.0, -27.5
Ae = 100 SQ. FT.	+16.0, -20.2	+16.0, -24.7	+16.0, -29.8	+16.0, -24.7

COMPONENTS AND CLADDING PRESSURES, WALLS (PSF)		
Ae (EFFECTIVE AREA)	ZONE 4	ZONE 5
Ae = 10 SQ. FT.	+19.8, -21.5	+19.8, -26.6
Ae = 20 SQ. FT.	+18.9, -20.6	+18.9, -24.8
Ae = 50 SQ. FT.	+17.8, -19.4	+17.8, -22.4
Ae = 100 SQ. FT.	+16.9, -18.5	+16.9, -20.6

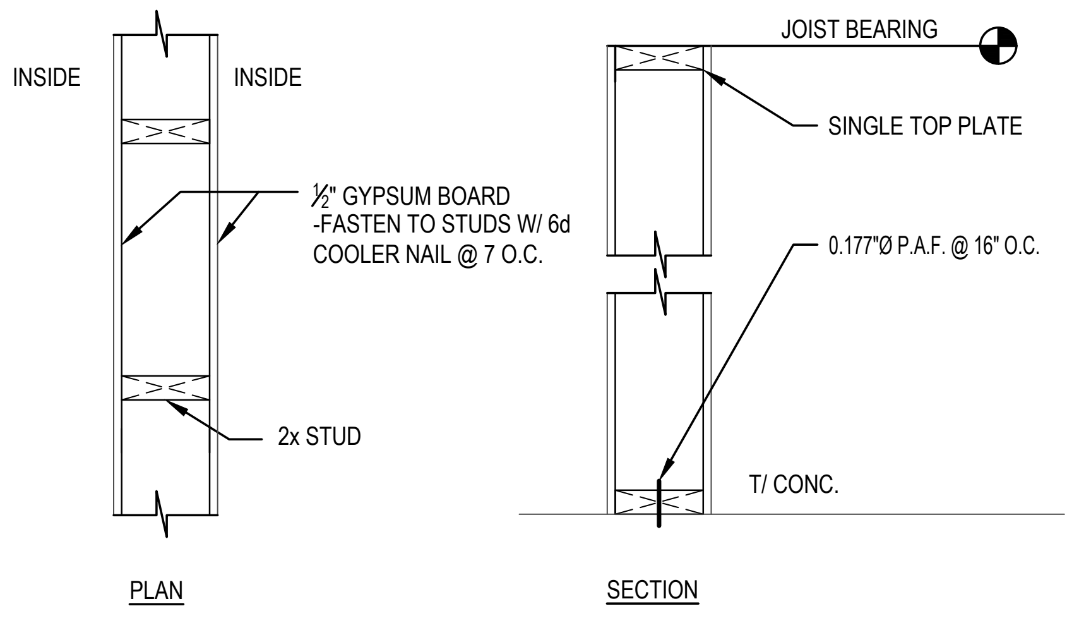
WOOD STRUCTURAL PANEL ROOF SHEATHING + NAILING SCHEDULE			ROOF FASTENING ZONES$^{(c)}$		
			①	②	③
SHEATHING THICKNESS	NAILS	PANEL LOCATION	FASTENING SCHEDULE (INCHES ON CENTER)		
½" OR LESS	8d COMMON	PANEL EDGE$^{(a)}$	6	4$^{(b)}$	3$^{(b)}$
		PANEL FIELD	6	6$^{(b)}$	3$^{(b)}$
19/32" OR GREATER	10d COMMON	PANEL EDGE$^{(a)}$	6	4$^{(b)}$	3$^{(b)}$
		PANEL FIELD	6	6$^{(b)}$	3$^{(b)}$

NOTES:
(a) EDGE SPACING ALSO APPLIES OVER ROOF FRAMING AT GABLE END WALLS.
(b) USE RING-SHANK NAILS IN THIS ZONE.
(c) ROOF FASTENING ZONES CORRESPOND TO WIND ZONE DIAGRAMS SHOWN ON S0.0.

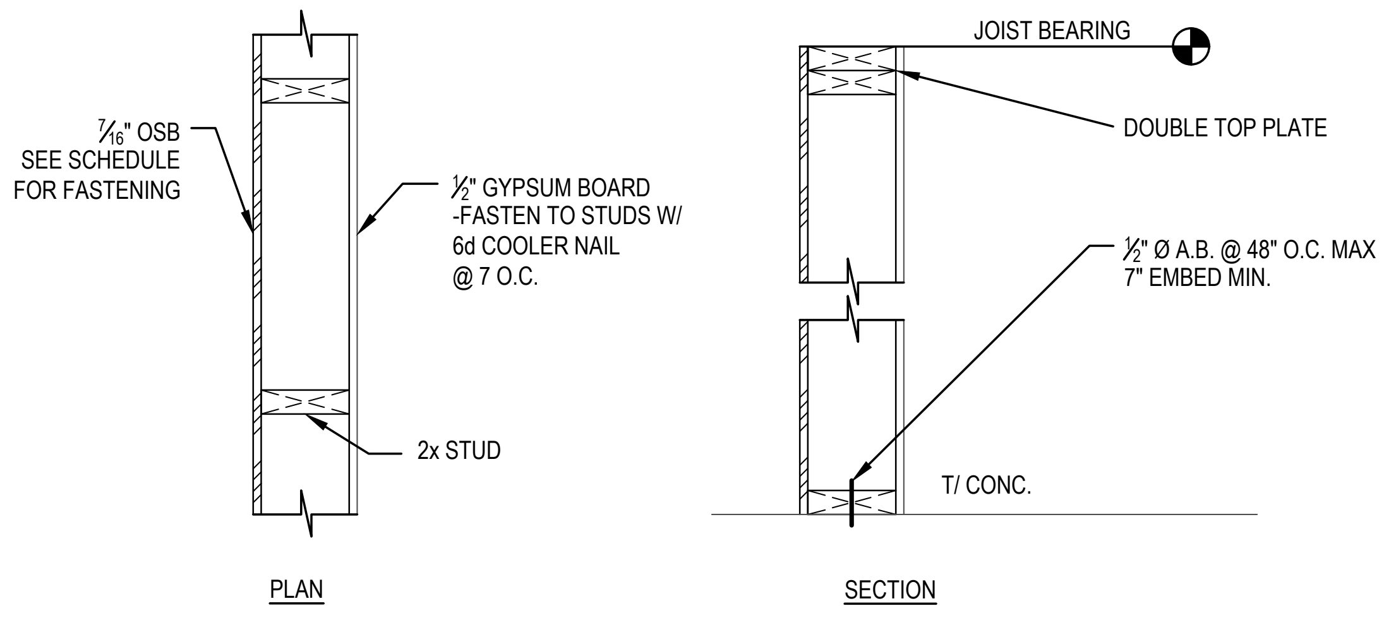
B
S-0.1

C&C DIAGRAMS, PRESSURES, AND FASTENING

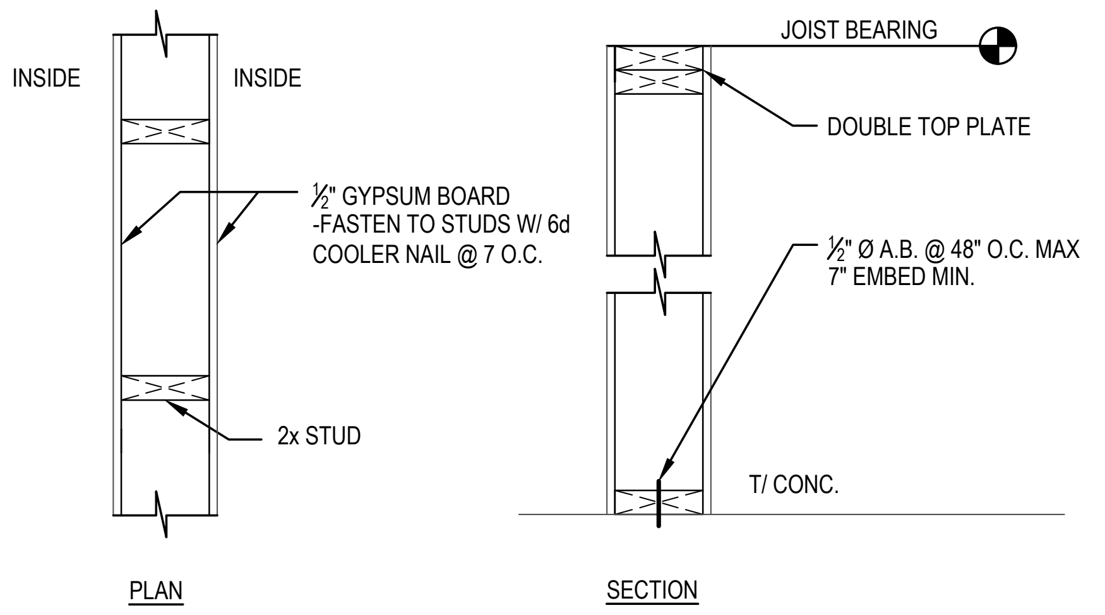
SCALE: N.T.S.



TYP. NON-LOAD BEARING, NON-SHEAR WALL



TYP. EXTERIOR WALL



TYP. INTERIOR BEARING WALL

C
S-0.1

WALL SHEATHING & ATTACHMENT DETAILS

SCALE: N.T.S.

NOTES:
1. 0.177"Ø POWDER ACTUATED FASTENERS TO HAVE 1½" MIN. PENETRATION.
2. ½"Ø ANCHORS TO HAVE 7" MIN. EMBEDMENT. CONTRACTOR OPTION: SUBSTITUTE ½"Ø SIMPSON TITEN HD W/ 5" EMBEDMENT.

No. 29213
PIERRE D. COIRON
4-10-24

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

GENERAL NOTES

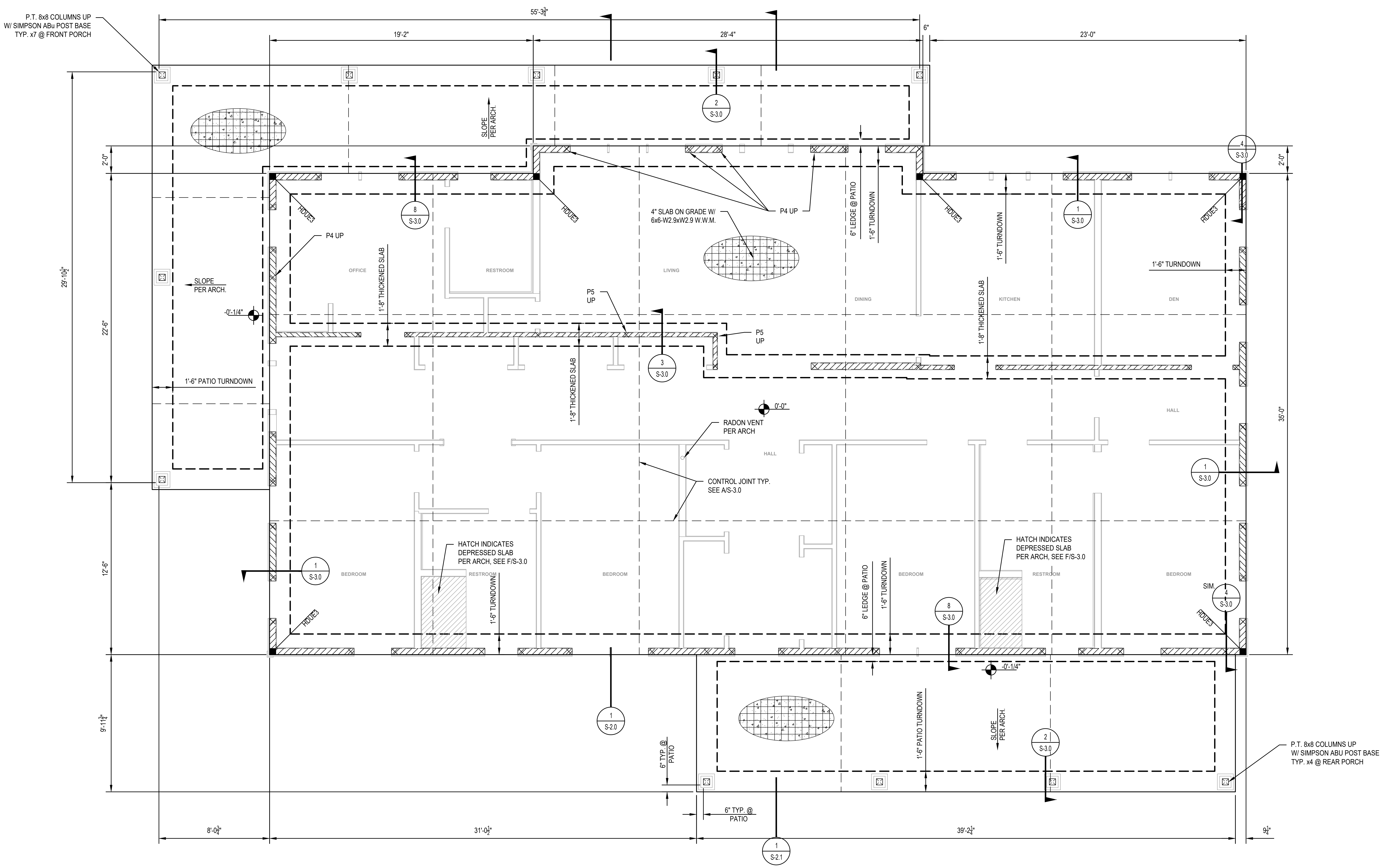
SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

S^e STABILITY ENGINEERING
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S–0.1
PROJECT NO. SE#26180
SHT 2 OF 8
REV 0

LEGEND	
✕	POST UP, P3 TYP. U.N.O.
○	POST DOWN, P3 TYP. U.N.O.
⊗	POST THRU, P3 TYP. U.N.O.
P3	(3) GANGED STUDS
P4	(4) GANGED STUDS
P5	(5) GANGED STUDS
▨	LOAD BEARING WALL
⊠	_x_ TIMBER POST
■	HOLD DOWN

26180 - CLARATEL CARE HOMES - OPTION 2.DWG



FOUNDATION PLAN

SCALE: ¼"=1'-0"

NOTES:

1. FOR GENERAL NOTES, SEE S-0.0 AND S-0.1
2. ALL DIMENSIONS TO F/WALL, E/CONC. OR ℄ POST. SEE ARCH. FOR DIMENSIONS NOT SHOWN.
3. ARCH. BACKGROUNDS SHOWN FOR GENERAL INFORMATION ONLY. SEE ARCH. FOR EXACT WALL LOCATIONS, DIMENSIONS, AND OTHER INFORMATION NOT SHOWN.

No. 29213
4-10-24
PIERRE D. COIRON

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

FOUNDATION PLAN

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

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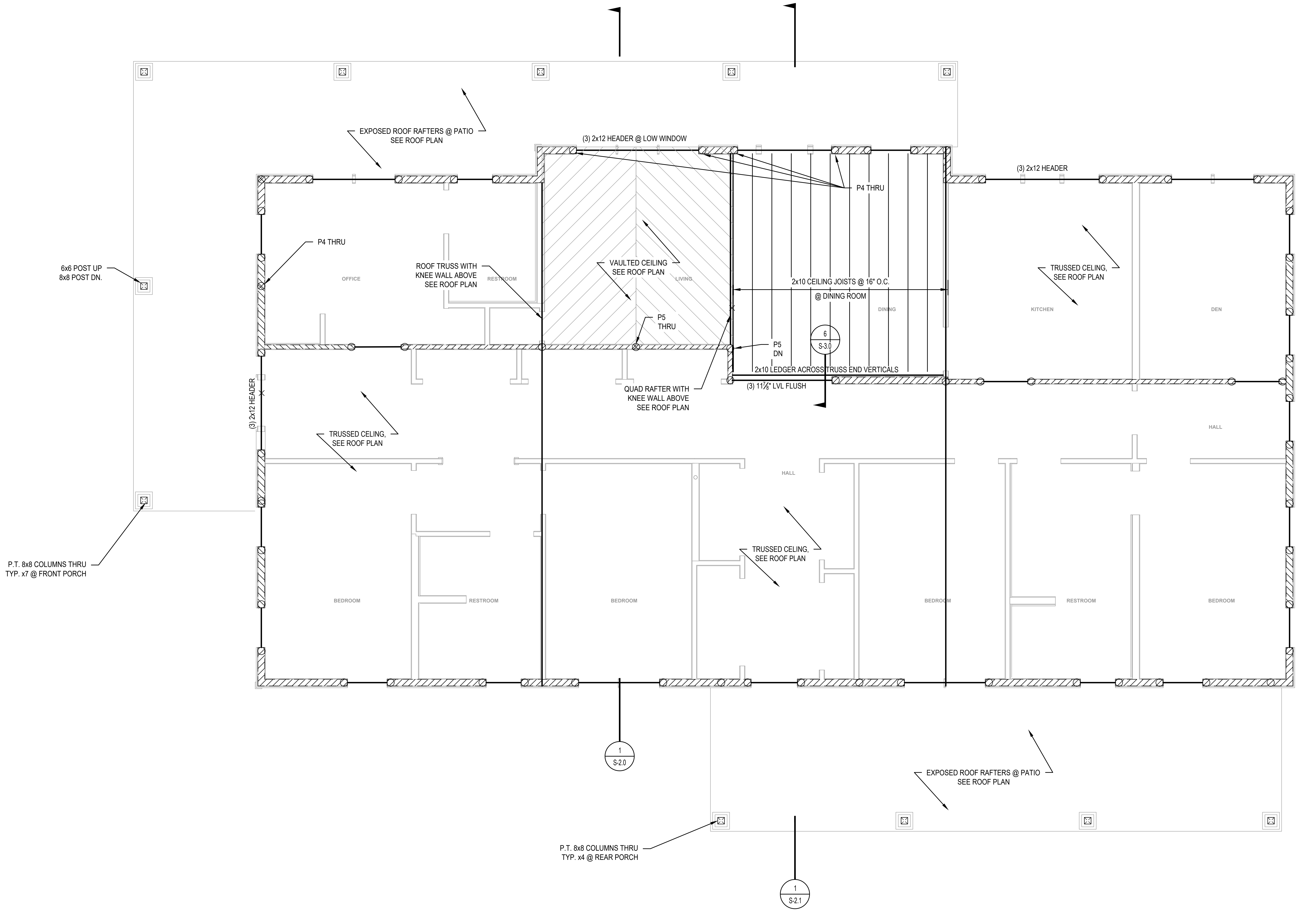
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S–1.0
PROJECT NO. SE#26180
SHT 3 OF 8
REV 0

LEGEND	
✕	POST UP, P3 TYP. U.N.O.
○	POST DOWN, P3 TYP. U.N.O.
⊗	POST THRU, P3 TYP. U.N.O.
P3	(3) GANGED STUDS
P4	(4) GANGED STUDS
P5	(5) GANGED STUDS
▨	LOAD BEARING WALL
⊠	_x_ TIMBER POST
■	HOLD DOWN

26180 - CLARATEL CARE HOMES - OPTION 2.DWG



CEILING FRAMING PLAN

SCALE: ¼"=1'-0"

NOTES:

1. FOR GENERAL NOTES, SEE S-0.0 AND S-0.1
2. FOR HEADERS NOT CALLED OUT, SEE SCHEDULE ON S-0.1
3. ALL DIMENSIONS TO F/WALL, E/CONC. OR ℄ POST. SEE ARCH. FOR DIMENSIONS NOT SHOWN.
4. ARCH. BACKGROUNDS SHOWN FOR GENERAL INFORMATION ONLY. SEE ARCH. FOR EXACT WALL LOCATIONS, DIMENSIONS, AND OTHER INFORMATION NOT SHOWN.

GEORGIA REGISTERED PROFESSIONAL ENGINEER
No. 29213
PIERRE D. COIRON
4-10-24

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

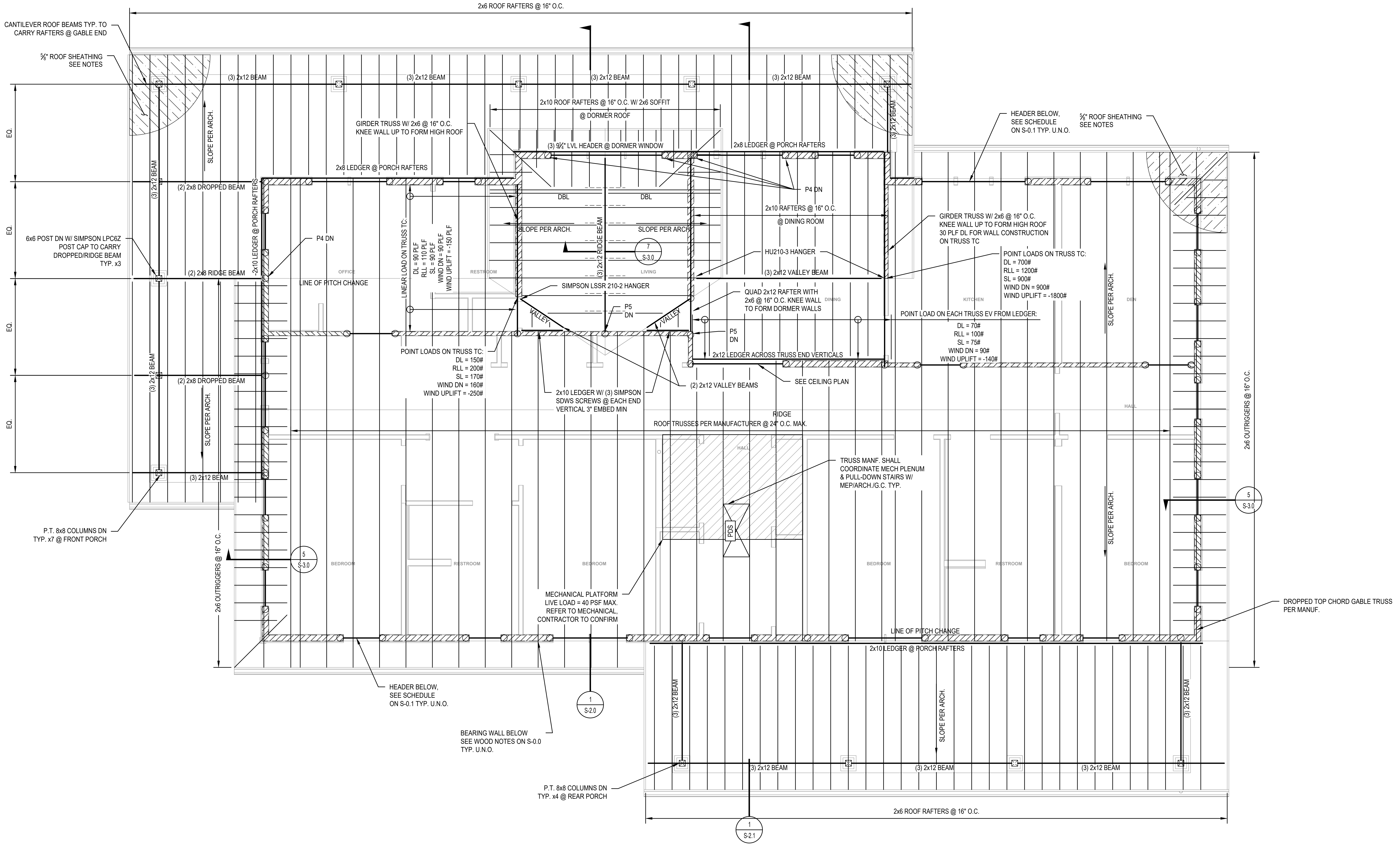
CEILING FRAMING PLAN

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

Se STABILITY ENGINEERING
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S–1.1
PROJECT NO. SE#26180
SHT 4 OF 8
REV 0

LEGEND	
╳	POST UP, P3 TYP. U.N.O.
○	POST DOWN, P3 TYP. U.N.O.
⊗	POST THRU, P3 TYP. U.N.O.
P3	(3) GANGED STUDS
P4	(4) GANGED STUDS
P5	(5) GANGED STUDS
▨	LOAD BEARING WALL
⊠	_x_ TIMBER POST
■	HOLD DOWN



ROOF FRAMING PLAN

SCALE: ¼"=1'-0"

NOTES:

1. FOR GENERAL NOTES, SEE S-0.0 AND S-0.1
2. FOR HEADERS NOT CALLED OUT, SEE SCHEDULE ON S-0.1
3. ALL DIMENSIONS TO F/WALL, E/CONC. OR ℄ POST. SEE ARCH. FOR DIMENSIONS NOT SHOWN.
4. ARCH. BACKGROUNDS SHOWN FOR GENERAL INFORMATION ONLY. SEE ARCH. FOR EXACT WALL LOCATIONS, DIMENSIONS, AND OTHER INFORMATION NOT SHOWN.
5. LOADS FOR TRUSS DESIGN CALLED OUT ON THIS PLAN ARE UNFACTORED.

No. 29213
PIERRE D. COTRON
4-10-24

ISSUED FOR CONSTRUCTION

26180 - CLARATEL CARE HOMES - OPTION 2.DWG

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

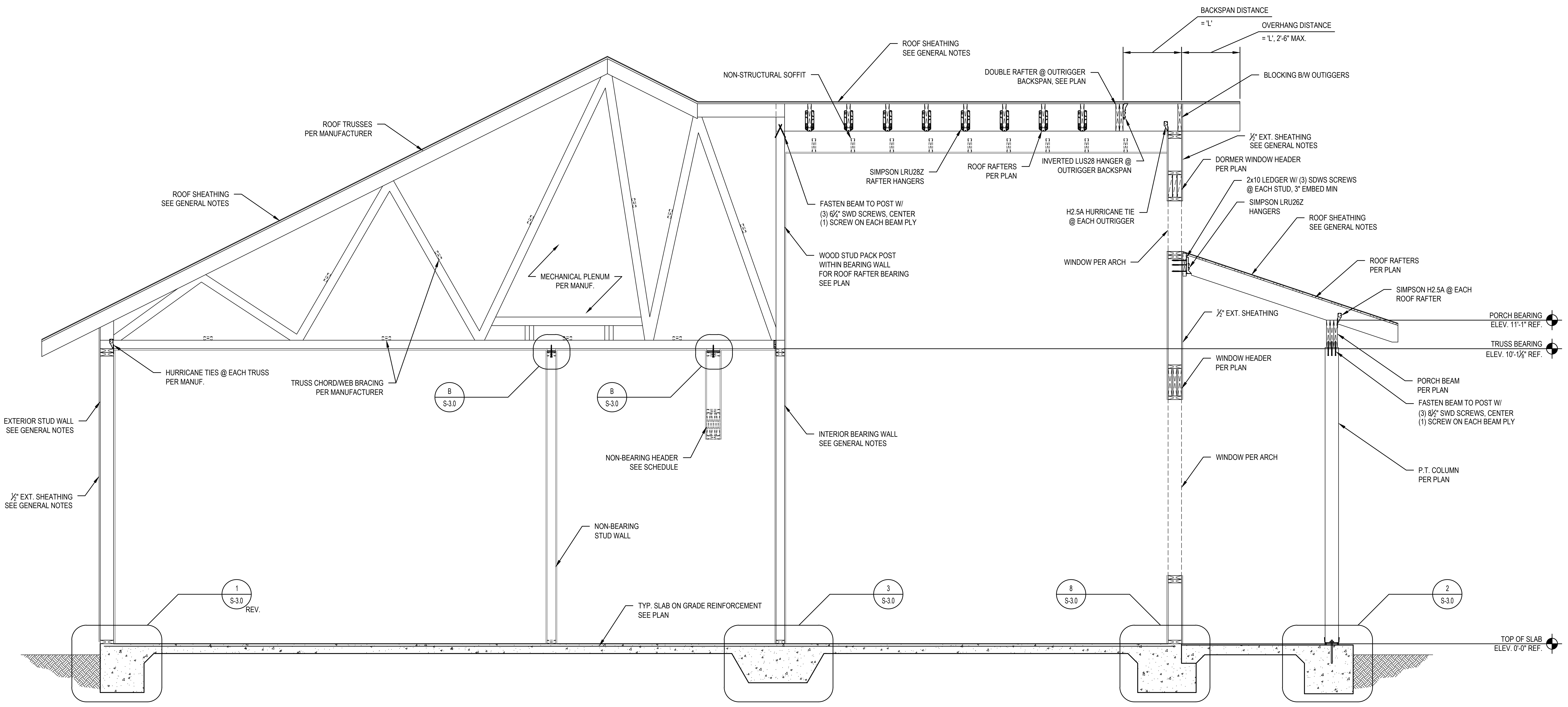
ROOF FRAMING PLAN

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

Stability Engineering
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S–1.2
PROJECT NO. SE#26180
SHT 5 OF 8
REV 0

26180 - CLARATEL CARE HOMES - OPTION 2.DWG



1
S-2.0
SECTION THRU DORMER
SCALE: 1/2"=1'-0"

PROFESSIONAL ENGINEER No. 29213
PIERRE D. COIRON
4-10-24

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

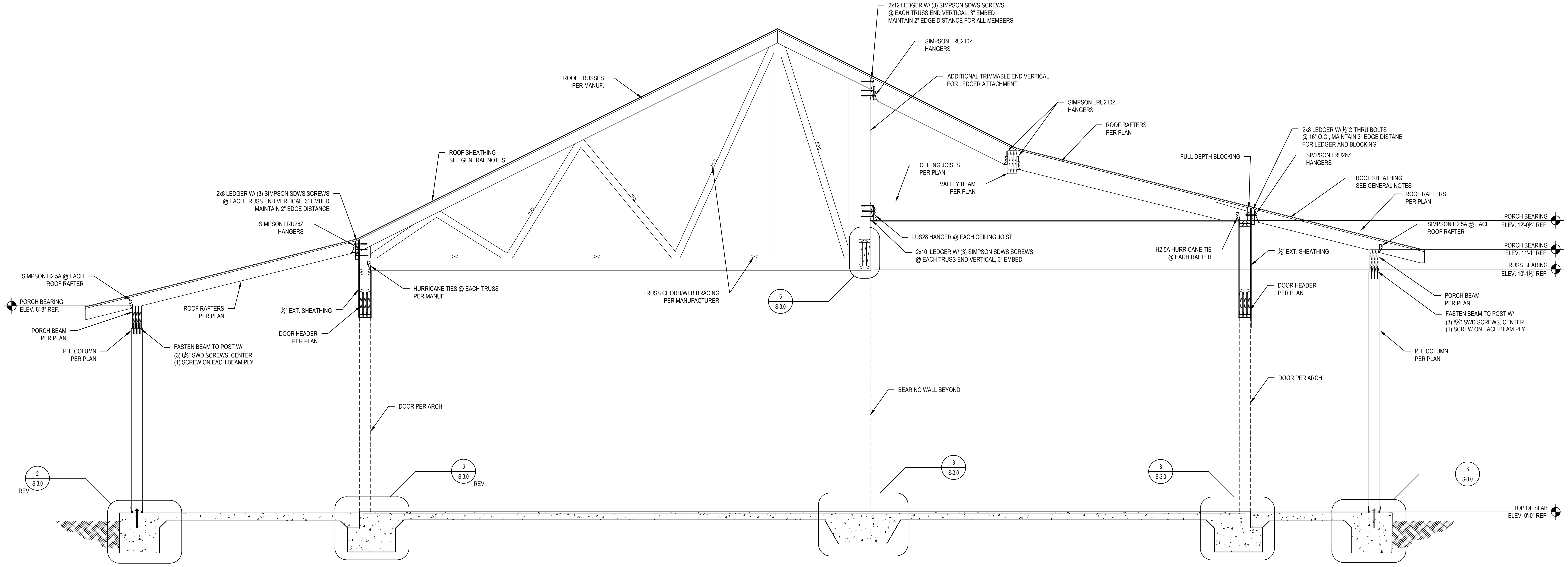
BUILDING SECTIONS

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: *JLJ*
CHKD BY: *YS*

S^e STABILITY ENGINEERING
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

SHEET NUMBER: S–2.0
PROJECT NO. *SE#26180*
SHT 6 OF 8
REV 0

26180 - CLARATEL CARE HOMES - OPTION 2.DWG



1 S-2.1 **SECTION THRU FRONT AND REAR PORCH**

SCALE: 1/2"=1'-0"

No. 29213
PIERRE D. COIRON
4-10-24

ISSUED FOR CONSTRUCTION

Monday, April 13, 2026 BY JOSH JOHNSON

REV	DESCRIPTION	DRWN	APPR	DATE

CLARATEL CARE HOMES – OPTION 2

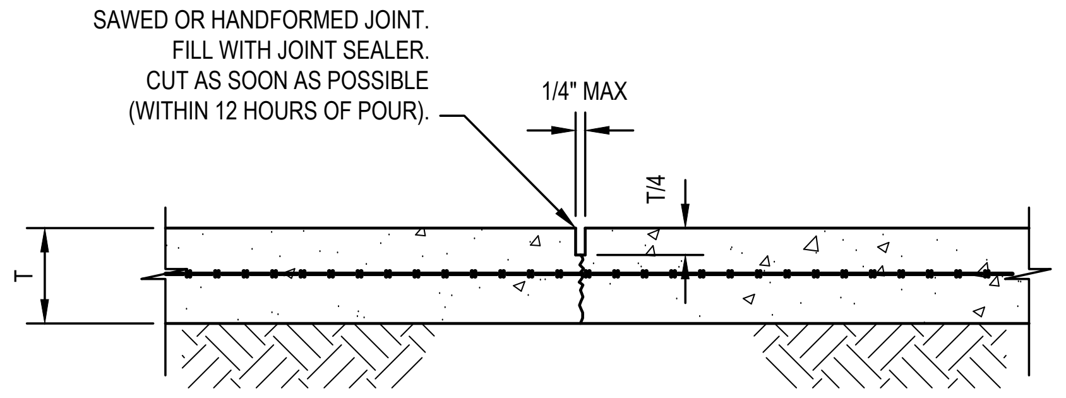
BUILDING SECTIONS

SCALE: NOTED
DATE: 04/10/2026
DRWN BY: JLJ
CHKD BY: YS

Se STABILITY ENGINEERING
1376 Church St, Ste 200, Decatur, GA 30030 Ph/Fax: 404-377-9316

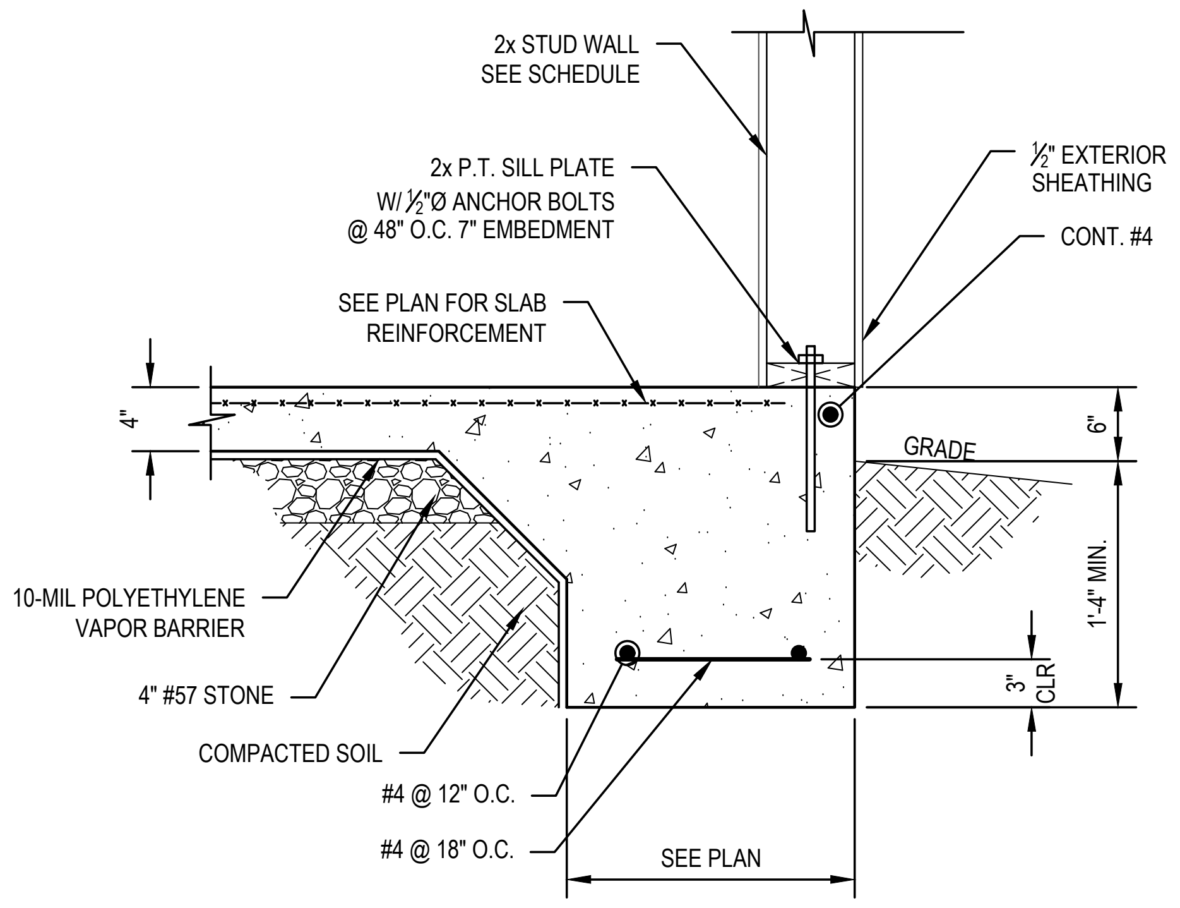
SHEET NUMBER: S–2.1
PROJECT NO. SE#26180
SHT 7 OF 8
REV 0

26180 - CLARATEL CARE HOMES - OPTION 2.DWG



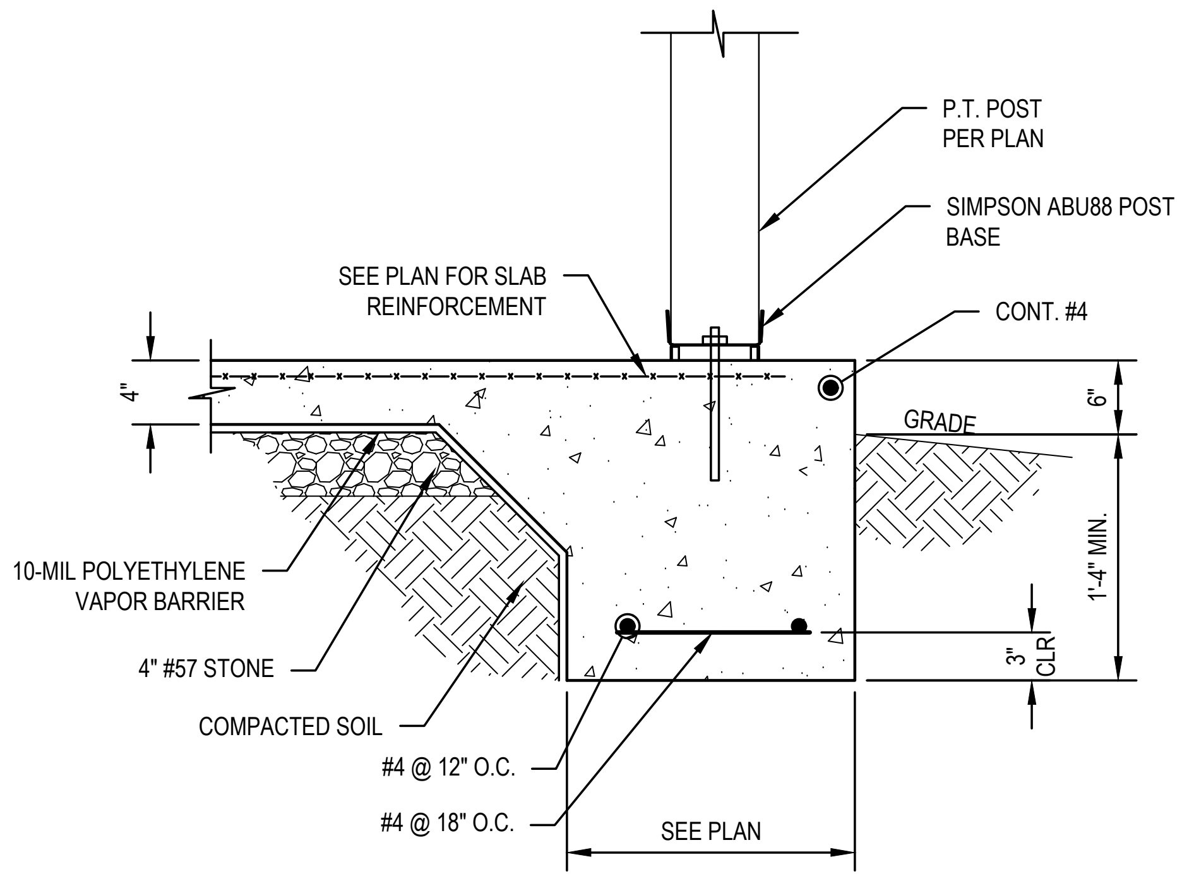
A / S-3.0 CONTROL JOINT FOR SLAB ON GRADE

SCALE: 1"=1'-0"



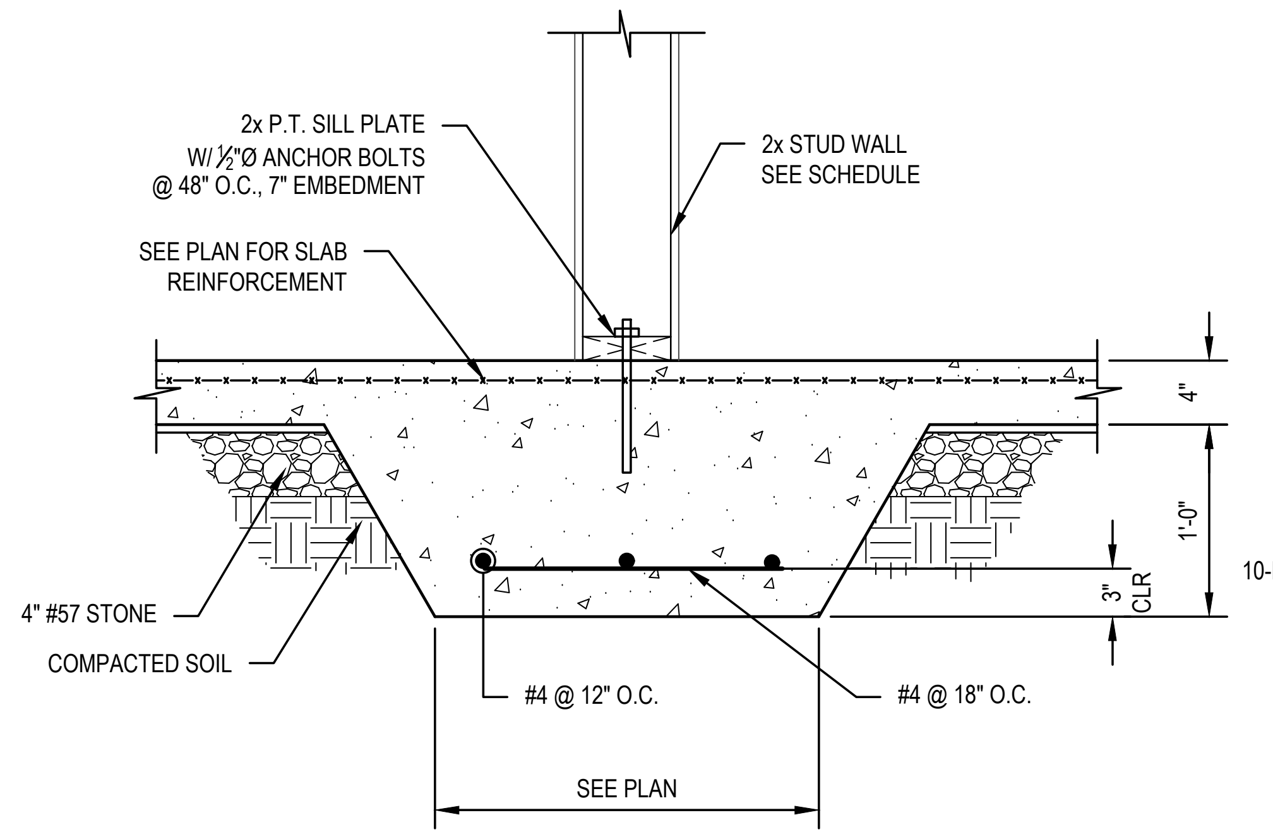
1 / S-3.0 TYP. TURNDOWN FOOTING

SCALE: 1"=1'-0"



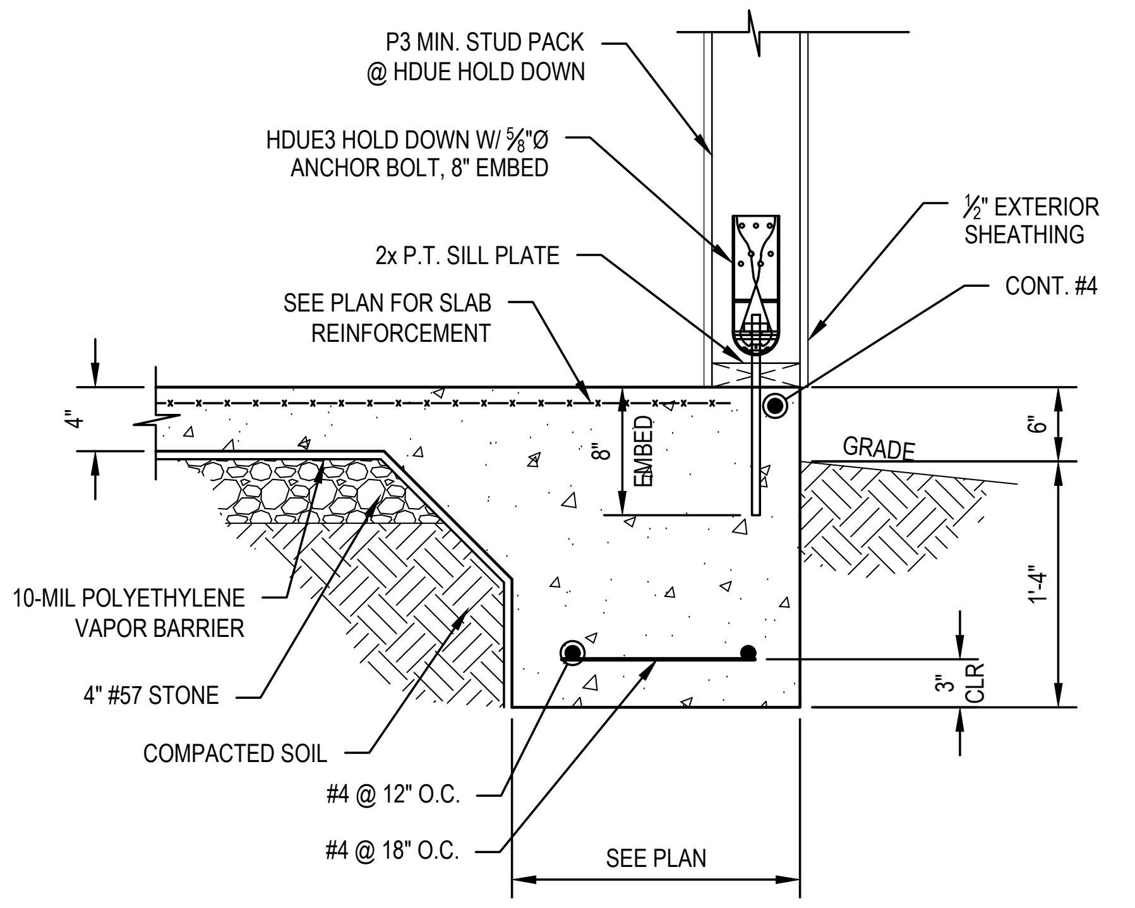
2 / S-3.0 TYP. PATIO FOOTING

SCALE: 1"=1'-0"



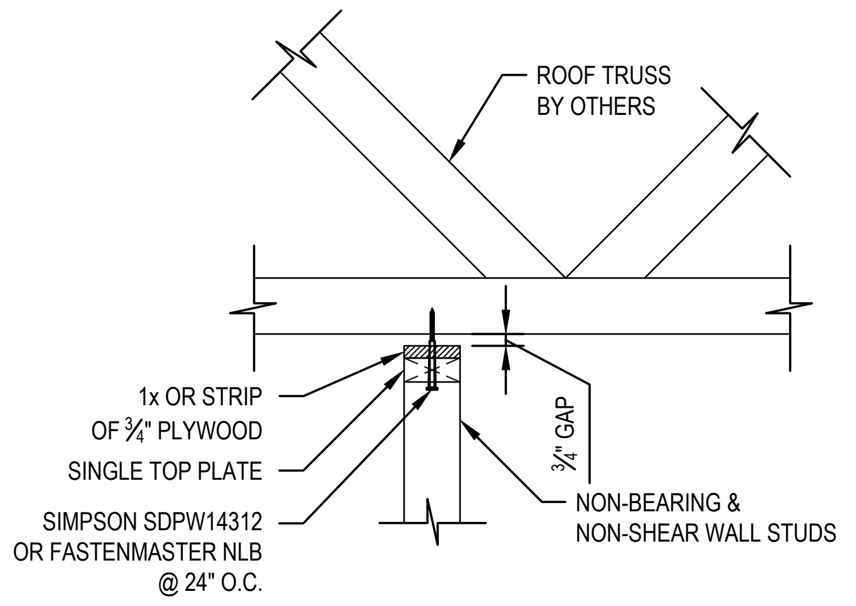
3 / S-3.0 TYP. THICKENED SLAB

SCALE: 1"=1'-0"



4 / S-3.0 TURNDOWN FOOTING @ HDUE HOLD DOWN

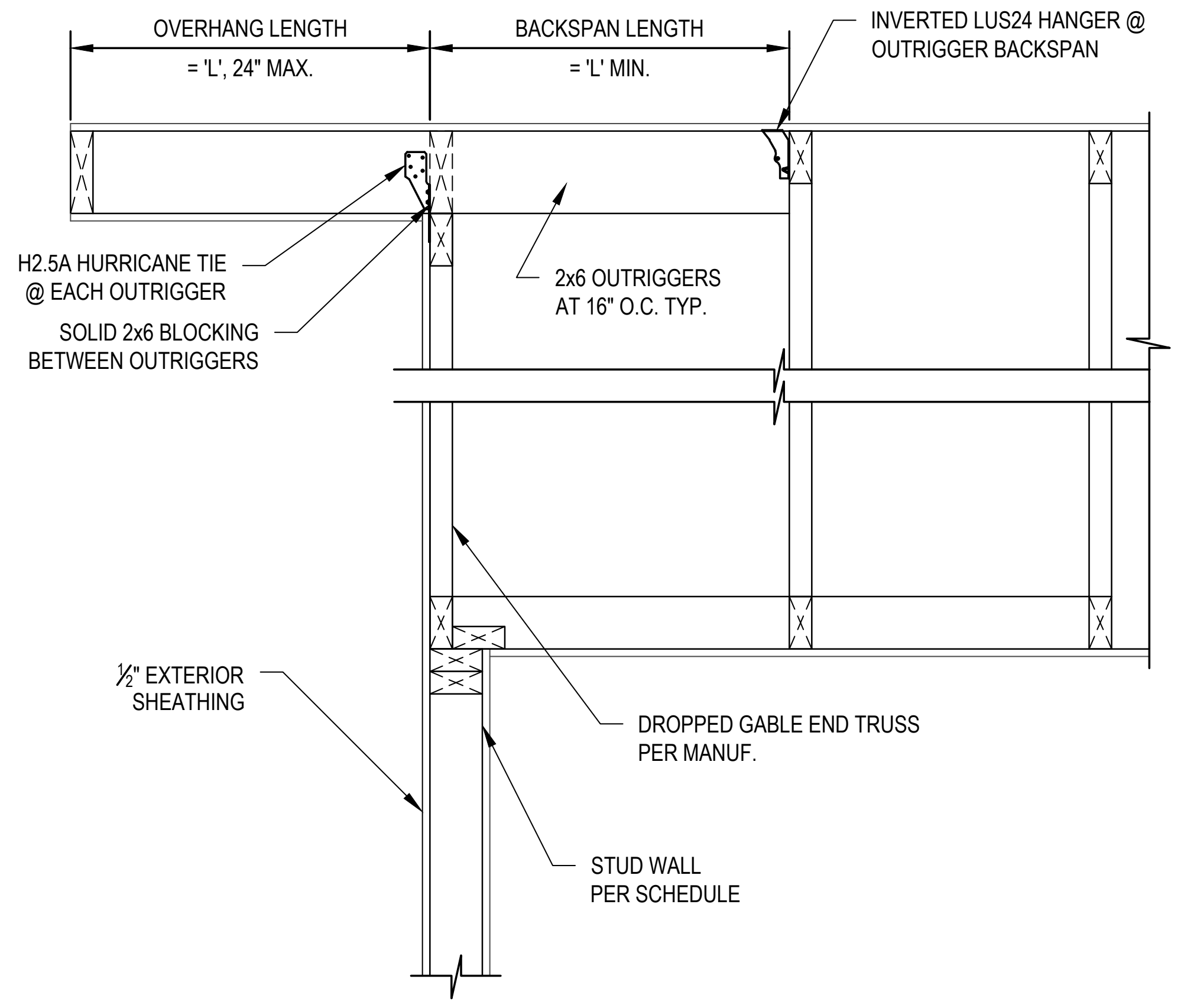
SCALE: 1"=1'-0"



B / S-3.0 NON-LOAD BEARING STUD WALL TOP CONX.

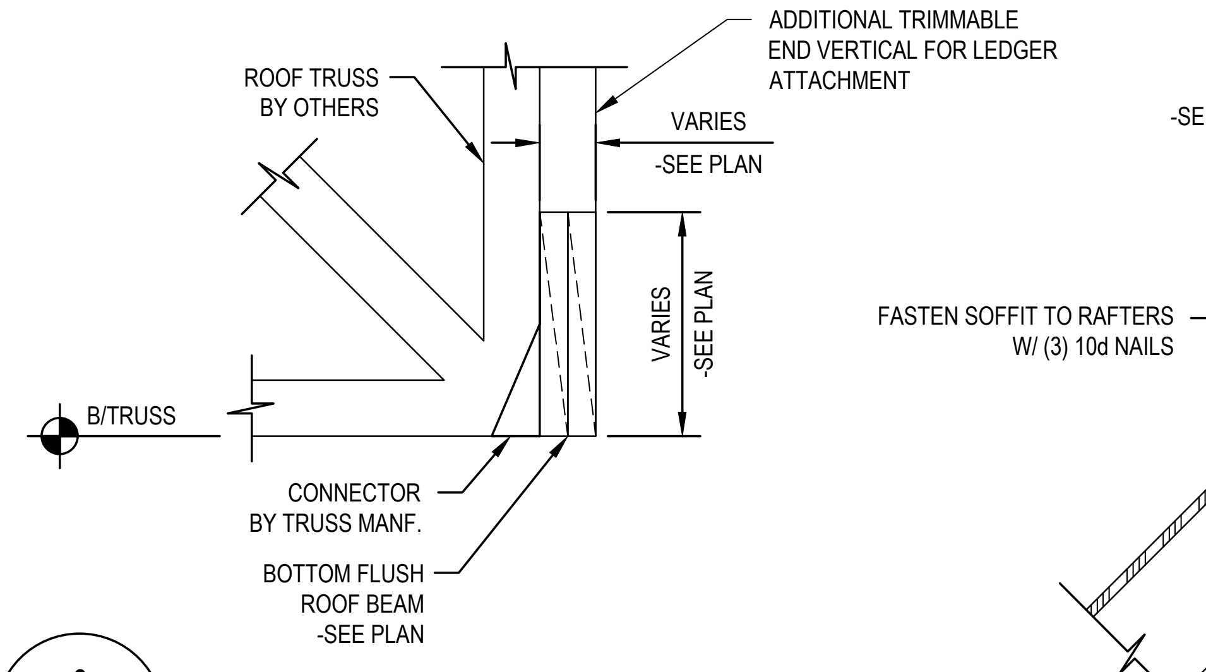
SCALE: 1"=1'-0"

NOTE: TRUSS PERPENDICULAR CONDITION SHOWN. AT TRUSS PARALLEL, PROVIDE MIN. 2x4 BLOCKING W/ (2) 10d TOENAILS EACH SIDE B/W TRUSS @ 24" O.C. MAX. FOR SCREW ATTACHMENT.



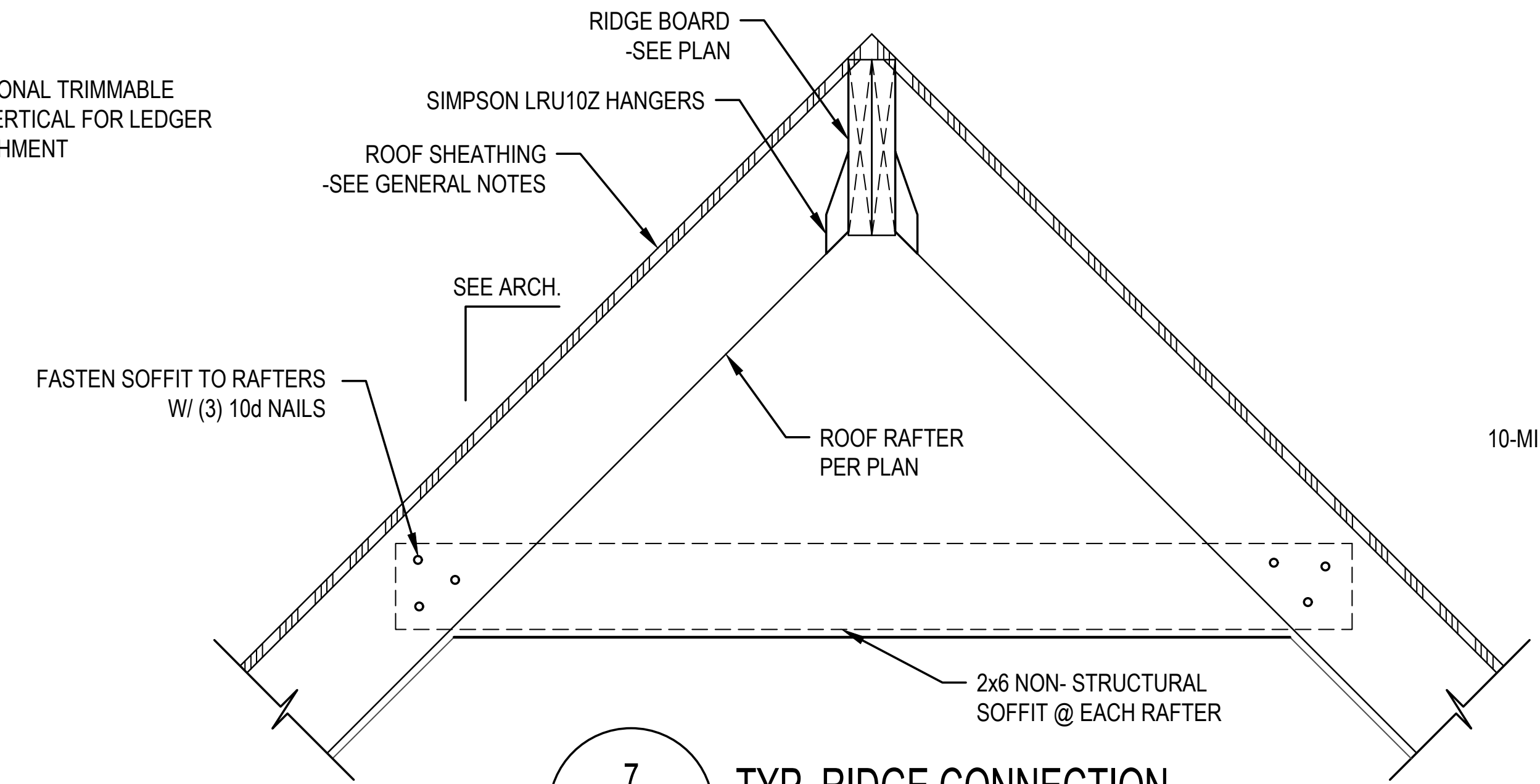
5 / S-3.0 GABLE END OUTRIGGERS

SCALE: 1"=1'-0"



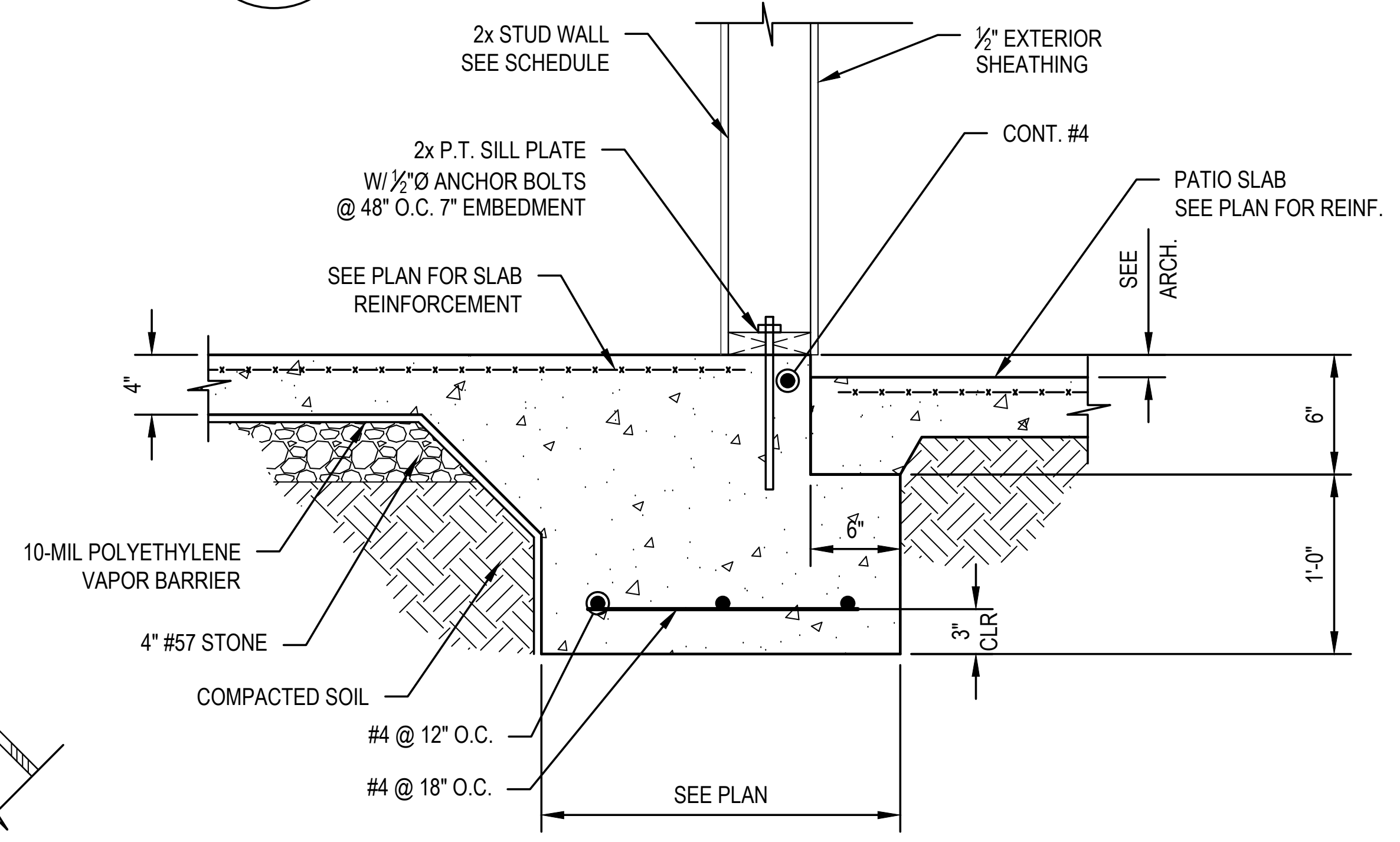
6 / S-3.0 TYP. BOTTOM FLUSH ROOF BEAM

SCALE: 1"=1'-0"



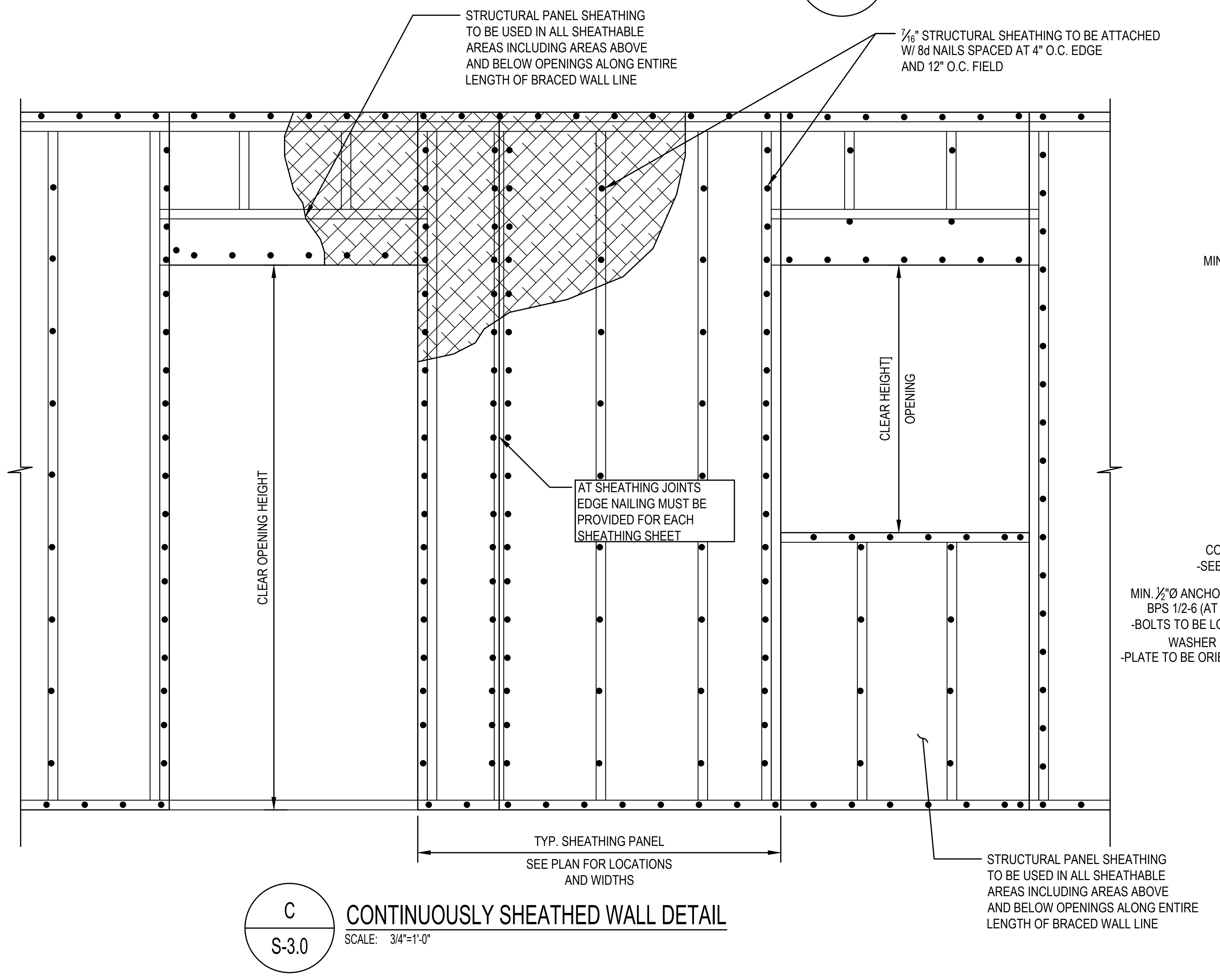
7 / S-3.0 TYP. RIDGE CONNECTION

SCALE: 1"=1'-0"



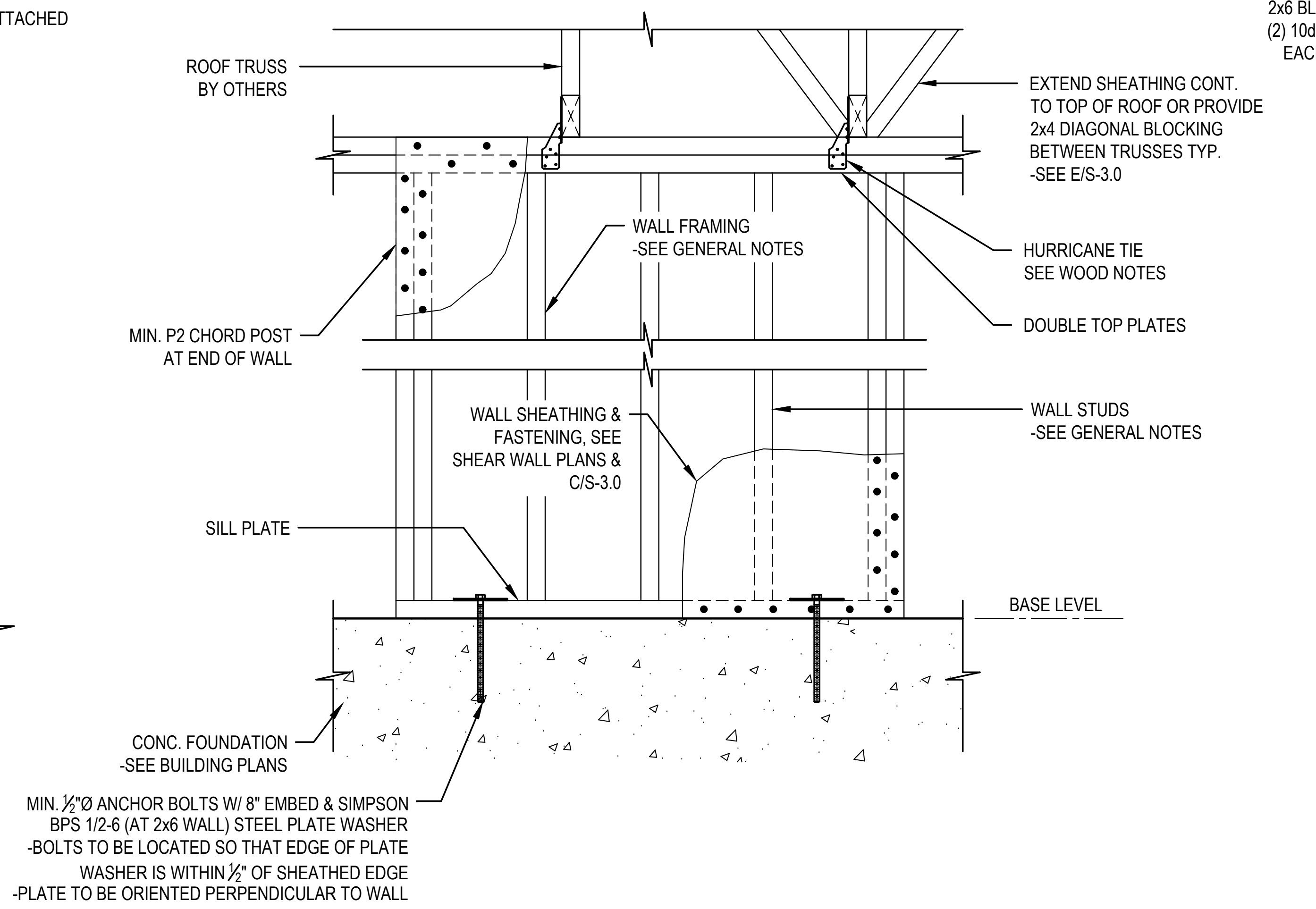
8 / S-3.0 TYP. TURNDOWN FOOTING @ PATIO SLAB

SCALE: 1"=1'-0"



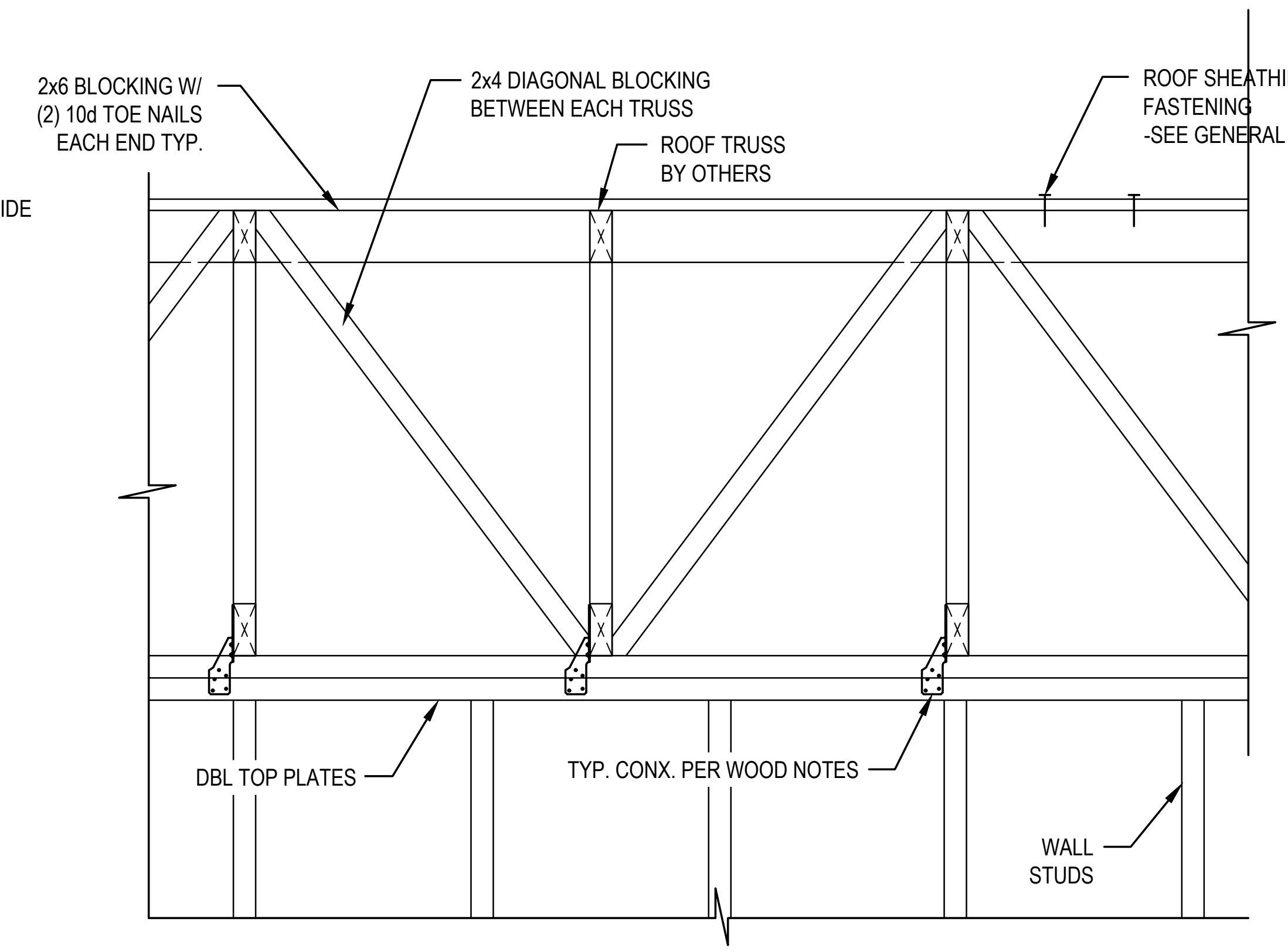
C / S-3.0 CONTINUOUSLY SHEATHED WALL DETAIL

SCALE: 3/4"=1'-0"



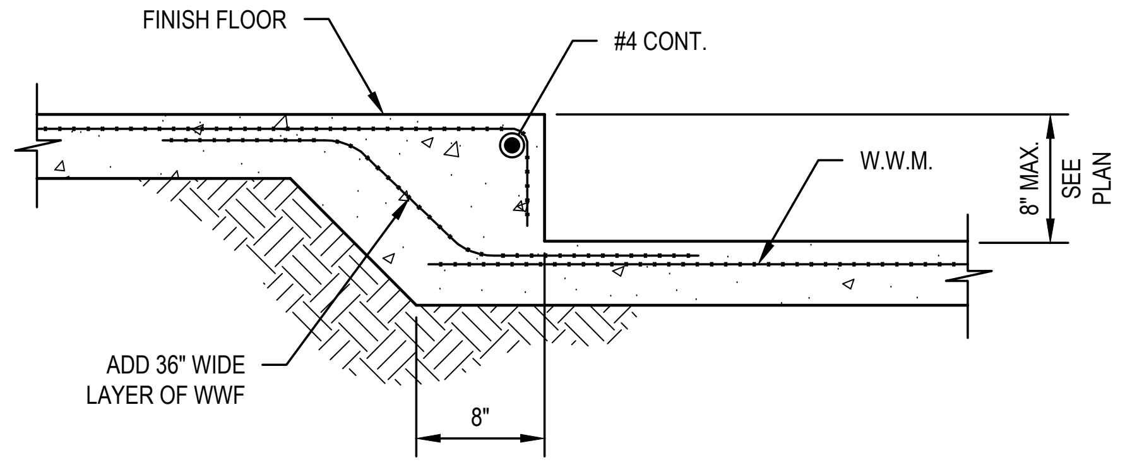
D / S-3.0 TYP. SHEARWALL DETAIL

SCALE: 1"=1'-0"



E / S-3.0 SHEARWALL BLOCKING AT ROOF SHEATHING NOT CONT. TO ROOF DECK

SCALE: 1"=1'-0"



F / S-3.0 TYP. DEPRESSED SLAB

SCALE: 1"=1'-0"

No. 29213
PROFESSIONAL
4-10-24
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CLARATEL CARE HOMES – OPTION 2

DETAILS

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SHEET NUMBER: S–3.0
PROJECT NO. SE#26180
SHT 8 OF 8
REV 0